

Fingal County Council Baldoyle Community Centre









Fingal County Council Baldoyle Community Centre

Engineering Report for Planning

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1. INTRODUCTION

This report has been prepared as part of a planning package submitted on behalf of Fingal County Council regarding the proposed development of a new Community Centre at Baldoyle Racecourse Park, Fingal. This report sets out the Engineering basis for the planning stage design of the scheme in terms of surface drainage, foul drainage, and water supply. This report should be read in conjunction with the following drawings and documents:

•	BCC-ROD-00-XX-DR-C-0001	Existing Site Layout
•	BCC-ROD-00-XX-DR-C-0005	Proposed Site Layout
•	BCC-ROD-00-XX-DR-C-0030	Existing Drainage Layout
•	BCC-ROD-00-XX-DR-C-0031	Proposed Drainage Layout
•	BCC-ROD-00-XX-DR-C-0040	Existing Watermain Layout
•	BCC-ROD-00-XX-DR-C-0041	Proposed Watermain Layout

2. PROPOSED DEVELOPMENT

The proposed development includes the delivery of a new community centre including sports hall, studies, changing rooms and toilet facilities. The development includes the reconstruction of existing parking located to the north of the site, landscaping of the surrounding area within the development site and associated utilities & drainage work.

3. SITE INFORMATION

3.1 Site Location

The site is located off Red Arches Road adjacent to Baldoyle Racecourse Park, Baldoyle, Fingal. The site is bound by the park on three sides, to the south, east and west and Red Arches Road to the north. Access to the Coast Road is gained approximately 400m to the east, and high-density residential developments less than 100m to the west. Refer to Figure 3.1 below for the proposed site location.



Figure 3.1: Site Location

Refer to Appendix A for Existing Site Layout.

3.2 Site Topography

The site is generally flat, with a slight slope from northeast to southwest. The levels across the site range from 4.18MOD at the entrance, to 4.72MOD in the east and 4.24MOD in the south-west. An existing disused building is present to the west of the site, which is accessed via an elevated pedestrian ramp which is currently fenced off. Existing carparking is present to the northwest of the site which is to remain. The existing parking in the centre of the site will be removed to facilitate the building construction.

Refer to Appendix C for Topographical & Utility Survey.

4. WATER SUPPLY

4.1 Existing Water Supply

The existing building is connected to the existing watermain network via a connection from the north to the north elevation of the building. There is a second connection located to the north-east of the building to a hydrant located on the raised footway. Both connections exit the site to the north across Red Arches Road to the existing public watermain located in the footway/cycleway. The size of the watermain is to be confirmed, as this information was not presented as part of the utility survey.

Refer to Appendix B for Drainage and Watermain Records.

Refer to Appendix C for Topographical & Utility Survey.

Refer to Drawings BCC-ROD-00-XX-DR-C-0040 & 0041 for the Existing & Proposed Watermain Layout.

4.2 Proposed Water Supply

The existing connection is to be utilised as part of the works. A pre-connection enquiry has been submitted to Irish Water who have confirmed that the proposed connection is feasible. A Connection Application will be submitted to Irish Water post planning. The building will be connected via a new meter box to Irish Water details, the position of which will be confirmed during the connection application.

The peak daily demand has been calculated as 0.29l/sec.

All watermains have been designed in accordance with Irish Water's 'Code of Practice for Water Infrastructure' (Document No. IW-CDS-5020-03) and will be constructed in accordance with same and Irish Water 'Water Infrastructure Standard Details' (Document No. IW-CDS-5020-01).

Refer to Appendix D for Irish Water Confirmation of Feasibility.

Refer to Drawings BCC-ROD-00-XX-DR-C-0041 for the proposed watermain layout.

5. FOUL DRAINAGE

It is proposed to reconfigure and extend the existing separate foul drainage system within the site boundary and to use an existing outfall to discharge from the site.

5.1 Existing Foul Drainage

The site and the existing building are served by an existing 100mm diameter foul drain which outfalls to the west of the site to a larger 225mm diameter foul sewer network.

Refer to Appendix B for Drainage and Watermain Records.

Refer to Drawings BCC-ROD-00-XX-DR-C-0030 for the existing drainage layout.

5.2 Proposed Foul Drainage

The existing 100mm diameter foul drain and outfall will be used to serve the new development. There will be some modifications to the drainage lines within the site boundary to cater for the new pop-up locations.

The wastewater network has been designed in accordance with Irish Water's 'Code of Practice for Wastewater Infrastructure' (Document No. IW-CDS-5030-03) and will be constructed in accordance with same and Irish Water 'Water Infrastructure Standard Details' (Document No. IW-CDS-5030-01).

Refer to Drawings BCC-ROD-00-XX-DR-C-0031 for the proposed drainage layout.

6. SURFACE WATER DRAINAGE

It is proposed to provide a new separate surface system to serve the development. This section outlines the existing surface water drainage services surrounding the site and gives our proposals for the surface water drainage requirements as part of the development.

6.1 Existing Surface Water Drainage

There is an existing surface water drainage network located within the site which includes a series of gullies and manholes. This outfalls from the site access to a main surface water sewer located at Red Arches Road. The manhole that the existing network discharges to is identified as manhole S53 on FCC's 'Taking-In-Charge' drawing that is included in the appendices. Adjacent to this, there is another manhole on a separate line, S56, which is also in the charge of FCC. Both lines, that are in the charge of FCC, subsequently flow to the north. No attenuation was recorded on site from the utility survey. An existing petrol interceptor is located adjacent the existing carpark before the outfall to the main sewer.

Refer to Appendix B for Drainage and Watermain Records.
Refer to Appendix I for Fingal County Council's Taking-In-Charge drawings.

Refer to Drawing BCC-ROD-00-XX-DR-C-0030 for existing drainage layout.

6.2 Proposed Surface Water Drainage

It is proposed to construct a new surface water drainage system for the building and carpark, however, this system will utilise the existing connection to the manhole at Red Arches Road which is in the charge of FCC.

The site will be attenuated for the 1-in-100 year + 20% climate change event. It has been calculated that 62.49m³ of combined on site storage will allow the site to achieve a greenfield discharge rate of 2 L/s without experiencing any flooding on site within the 1 in 100 year + 20% climate change. This is being incorporated through a mixture of underground storage in the form of pipework and permeable paving or grasscrete, above ground attenuation in the form of grassed rain gardens.

The use of soakaways has been confirmed as unfeasible following a ground investigation, for the purpose of these calculations no permeability of the soil has been used to determine the maximum attenuation required. A breakdown of the SUDS features is illustrated below in Table 6.2.

SUDS Storage Features		m ³ Storage
Permeable carpark	Area 1	5.01 m ³
	Area 2	4.98 m ³
Above ground attenuation	Area 1	25.5 m ³
	Area 2	9.3 m ³
	Area 3	17.7 m ³
	Total	62.49 m ³

Table 6.2: SUDS Storage Features

The stormwater system is designed for 50mm/hr rainfall intensity as per Greater Dublin Strategic Drainage Study (GDSDS). They were designed for a minimum self-cleansing velocity of 0.7m/s in accordance with BS EN 16933-2:2017 and the Greater Dublin Strategic Drainage Strategy. However, following consultation with Fingal

County Council Drainage department, an allowance for climate change of 20% was included rather than the 10% allowance called up in the GDSDS.

Refer to Appendix G for Surface Water Calculations.

Refer to Drawings BCC-ROD-00-XX-DR-C-0031 for proposed drainage layout.

6.2.1 SUDS Approach

This should be read in conjunction with the following:

(i) BCC-ROD-00-XX-DR-C-0031

The proposed SuDS features will include a combination of Source Control, Site Control and Regional Control measures as part of a Management Train whereby the surface water is managed locally in small sub-catchment rather than being conveyed to and managed in large systems further down the catchment.

Where possible the potential for surface water infiltration to the subsoil should always be utilised to help reduce the impact on the existing surface water drainage network downstream. However, a detailed ground investigation has not yet been carried out on the site. This proposed investigation will include for infiltration tests and should the ground prove suitable, this will also be incorporated into the system at detailed design stage.

It is proposed to provide the following SuDS measures:

- 1) Permeable Paving
- 2) Rain Garden with stone base below
- 3) Surface water pipes
- 4) Green roof
- 5) Petrol interceptor

Attenuation

Attenuation will be provided beneath three number rain gardens/soakaways located to the west, south and east of the proposed building. These grassed areas will act as attenuation for the site, allowing surface water to accumulate to a maximum high water level during a 1 in 100 year + 20% storm event. The rain water will continue to the discharge manhole to the north of the site, where the flow will be restricted to a green field run off rate of 2 L/s.

While it is acknowledged that Fingal County Council discourage the use of underground attenuation on sites, it is noted that the site in this case is extremely tight and that it is simply not possible to meet the storage requirements using other methods alone.

Grasscrete Paving

It is proposed that grasscrete paving will be used below the car parking area. Design of which will be subject to further design at detailed design stage. The paving will act as temporary storage for rainwater from the parking and roads to the north of the site. Grasscrete paving will also act to reduce the discharge of oil spillages from the site, as the primary area where vehicle movements are experienced are to be drained via permeable paving. A petrol interceptor is being provided as an additional means of reducing the discharge of oils and spillages from the site.

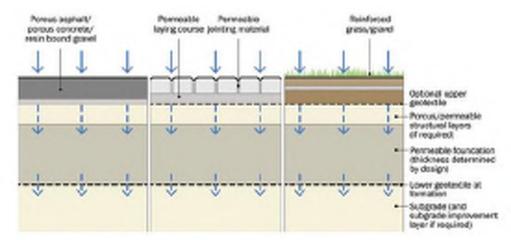


Figure 6.2 'SUDs Manual Figure 20.14 Pervious Pavement System Type A- Total Infiltration'

Green Roof

It is intended for a portion of the community centre to feature a green roof. This will assist in reducing the run off rate from the roof and to act as treatment for the storm water. The green roof design will be subject to further design at detailed design stage. The roof will not act as storage for water on the roof but an allowance for impermeability has been taken as per the calculations.

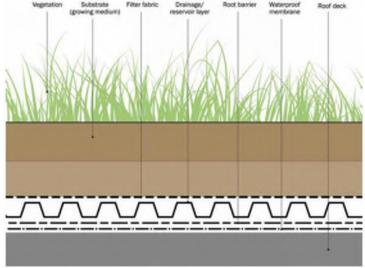


Figure 6.4 'SUDs Manual Figure 12.1 Section through a typical green roof'

Impermeability Factors

The impermeability factors of 0.6 for streets and footways, 0.6 for green roof and 0.1 for grassed areas are chosen in line with Table 26.14 'Impermeability and pollution indices for different land use types' of the SUDs Manual. This table has been reproduced below.

TABL 26.1

Land use surface type (LUST)	Impermeability (IMP _{RF})	Total suspended soluds pollution index (PI _{TSS})	Organic pollution index (Pl _{org})	Hydrocarbon pollution index (PI _{PAH})	Metals pollution index (PI _{HM})
Roofs					
industrial/commercial	1.0	0.3	0.3-0.4	0.2	0.4-0.8
residential	0.9	0.4-0.5	0.6-0.7	0.1	0.2-0.5
Highways					
motorways	0.8-0.9	0.9	0.7	0.9	0.8
major arterial highways	0.7-0.8	0.8	0.7	0.8	0.8
urban distributor roads	0.6-0.7	0.7-0.8	0.5	0.8	0.7
residential streets	0.4-0.6	0.4	0.6	0.6	0.6
- pavements	0.5-0.6	0.4	0.6	0.3	0.3
Car parks/hardstanding					
industrial/commercial	0.6-0.8	0.6-0.7	0.6-0.7	0.7	0.4-0.5
driveways (residential)	0.5	0.5	0.6	0.4	0.3
Open areas					
gardens (all types)	0.1	0.3	0.2-0.3	0	0.01
parks/golf courses	0.2	0.2-0.3	0.2	0	0.02
grassed areas (including verges, all types)	0.1	0.2-0.3	0.2-0.3	0.05	0.05

Note

Figure 6.5 'SUDs Manual Table 26.14 Impermeability and pollution indices for different land use type'

7. FLOOD RISK ASSESSMENT

A flood risk assessment was carried out as part of the preliminary design of the project, this included a review of the following flood risks present for the site;

- Tidal; flooding from high sea levels
- Fluvial; flooding from water courses
- Pluvial; flooding from rain & surface water
- Development surcharges; flooding from existing surface water sewers

The low risk level associated with these items is detailed in the sections below.

Furthermore, the flood maps produced by McCloy Consulting as part of the *Stage 3 Material Alterations to the Draft Fingal County Development Plan 2023-2029* indicate that the site is within Flood Zone C (low risk >0.1% AEP) for the current and midrange scenarios.

Refer to Appendix H for OPW and McCloy Consulting flood risk mapping. Refer to Appendix I for Fingal County Council's Taking-In-Charge drawings. Refer to Drawings BCC-ROD-00-XX-DR-C-0031 for proposed drainage layout

7.1 Tidal

The site is located approximately 450m from the Irish Sea/Dublin Bay. The road level remains at approx. 5m MOD until the Coast Road, after which the verge slopes down to sea level.

The Dublin Coastal Protection Project indicated a 2002 high tide event where the water level reached 2.95 MOD. The proposed finished floor level of the community centre is at 4.8m, 1.85m above this high-water level, therefore tidal flooding risk is considered to be low.

¹ Pollution index values are based on reported land use type EMC distributions and impact potential thresholds from House et al (1993), Luker and Montague (1994), Butler and Clark (1995), D'Arcy et al (2000), Mitchell (2005) and Moy et al (2003).

7.2 Fluvial

The site is located approximately 150m from a historic ditch which acts as a land runoff for the Mayne River which flows into Dublin Bay. The OPW fluvial flood extent map MAY/HPW/EXT/CURS/003 illustrates the closest reference point of 1Maa675 which extracts the following information.

Node Label	Water Level	Water Level	Water Level
	mOD	mOD	mOD
	10% AEP	1% AEP	0.1% AEP
1Maa675	2.50	2.85	3.46

Table 7.2; Fluvial Map Node information

As the proposed finished floor level is at 4.8m, the highest water level anticipated from fluvial flooding is 1.34m below the proposed finished floor level, therefore fluvial flood risk is considered low.

7.3 Pluvial

Previous flood events were analysed as part of the preliminary flood risk review. Two events were noted which were closest in proximity from the proposed site, located approximately 380-500m south. These events occurred in 2011 & 2002, with both events being noted as a result of excessive rainfall and the existing surface water systems being unable to accommodate the heavy rainfall. These events were not considered a risk to the proposed development due to the distance from each, and the proposed overground attenuation on the site which will act as storage for such heavy rainfall events.

7.4 Development Surcharges

The surrounding area was analysed in terms of development and existing surface water infrastructure surrounding the site.

There is existing FCC surface water drainage infrastructure in Red Arches Road with the closest manholes to the site being S53 & S56 indicated on FCC's TIC drawings. The OPW Flood Event Maps do not indicate any issues with this infrastructure. We met with FCC Drainage personnel on site on the 20th January 2022 who were not aware of any surcharge issues.

Baldoyle Racecourse park is adjacent the development site. Runoff from the park is not considered to be a risk as along the eastern perimeter of the site, the levels reduce to form a ditch and tree line between the site and the park. To the south and west of the site, the ground slopes away from the site. This combined with the proposed finished floor level of the building means that there is a low risk of flooding from surface water flows from the surrounding areas.

7.5 Surface Water Connection

The existing connection to the public surface water sewer is to be maintained. An underground CCTV survey is planned to be carried out at detailed design stage to ensure the existing connection is of a good state and the connection is to the existing manhole which discharges to the north. Following a site meeting with FCC Drainage personnel on the 20th January 2022, it was identified that maintaining the existing connection to the manhole in Red Arches Road (S53 on FCC's TIC drawings) was the most appropriate approach. The outfall from the proposed site will be reduced

considerably from the existing as the proposals include restricting the discharge from the site to 2 l/s and incorporating overground attenuation within the site.

7.6 Flood Risk Conclusion

Following the above analysis it is considered that the proposed development is not subject to an adverse risk of flooding based on the historic desktop survey carried out.

For fluvial and coastal flooding, the Flood Maps indicate that the site is within Flood Zone C (low risk >0.1% AEP).

For development surcharges, a detailed surface water analysis has been carried out for the proposed development which shows that the site is anticipated to experience no flooding in a 1 in 100 year plus 20% climate change storm event. In any case, should an extreme storm event occur, the overground attenuation areas will overflow, and surface water is directed away from the building to ensure the building does not experience flooding from surface water sources.

8. BUILDING

8.1 Structure

A superstructure scheme has been identified which is likely to be the most costeffective solution to accommodate the requirements of the curved roof profile with minimal structural depth, spanning over the large open sports hall.

The main steel structural frame will consist of a portal frame structure formed by columns and skewed rafters. Skewed rafters with varying alignment of the vertical member axis and with stepped heights have been adopted to facilitate the required roof curvature perpendicular to the span.

The span of the frames is approximately 18.0m (matching the width of the sports hall) and in the orthogonal direction, a bay spacing of 4.8m has been adopted.

8.2 Rafters & Columns

Rafters and columns with a 533x210x122 UB section size are envisaged. The rafter to column connection will be a moment connection providing lateral stability in this direction. The columns will be bolted to the RC roof slab of the surrounding low-rise areas, giving a column height of approximately 4m above this level.

The rafters will be tied-in the perpendicular direction by steel beams (139 CHS).

Trimmers for roof lights will consist of 203 UC 46 sections.

The frame will be provided with roof and longitudinal bracing which will provide the building's lateral stability in the orthogonal direction to the portal moment frames.

All the steel grades for the portal frame members will be grade class S355 and the required execution class will be Execution Class 3.

8.3 Surrounding Walls

The low-rise areas surrounding the sports hall will typically be formed using 215mm blockwork walls with an RC roof slab. Where a sloped roof is required to these areas (either end of sports hall) it is envisaged that a steelwork frame formed by 203 UC 46 sections will bolted to the RC roof slab at 4m c/c.

8.4 Substructure

The ground investigation works undertaken to date indicate the presence of Made Ground to depths of approximately 4m overlying a thin layer of soft organic silts / sands or gravels which extend to depths of between 4m and 4.80m. These strata are underlain by a very stiff to hard black gravelly clay and the boreholes all extended to the target depth of 10m in this very stiff Dublin Black Boulder Clay.

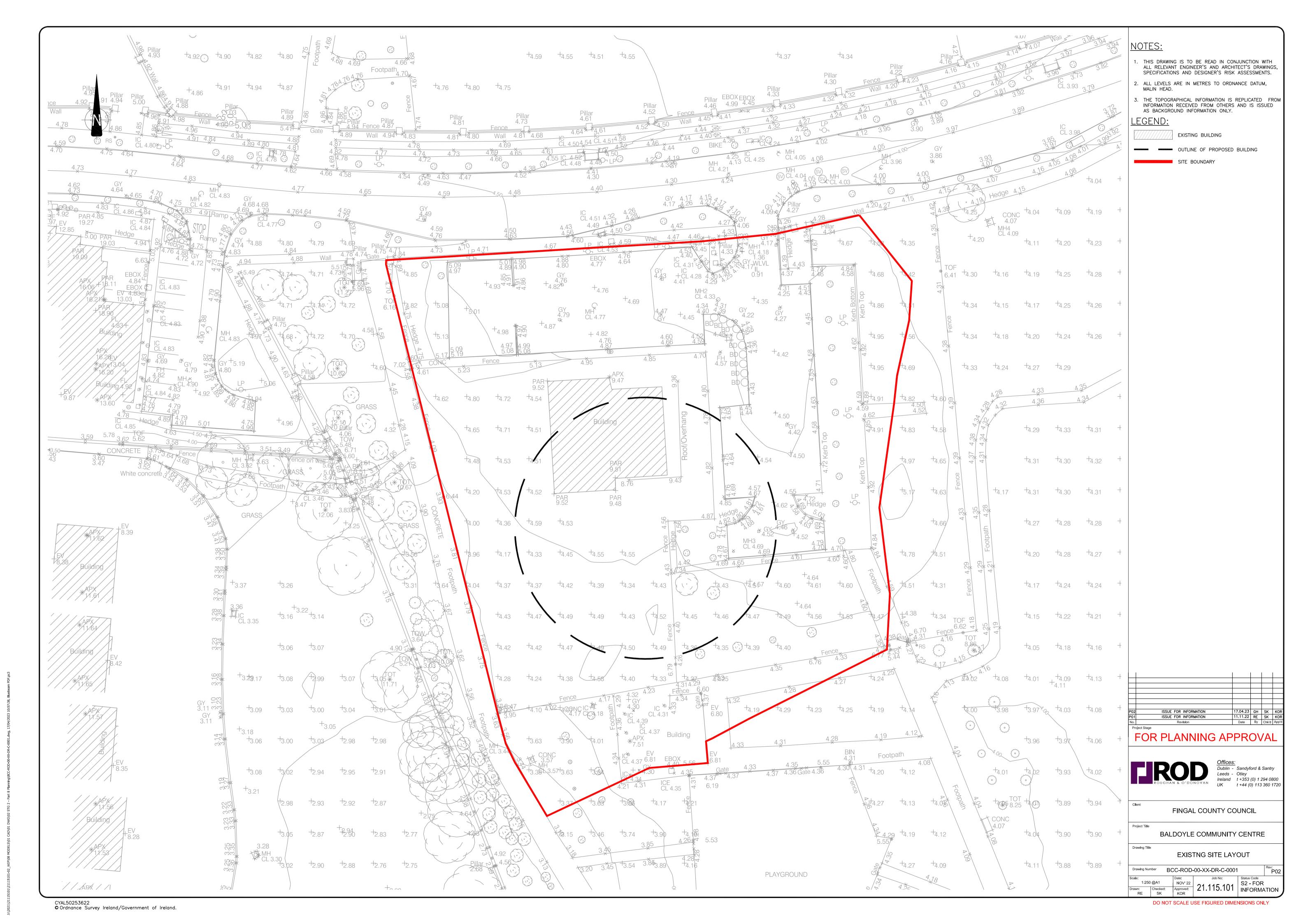
Foundations will have to be transferred to the black gravelly clays, probably by short driven piles, with pile depths of circa 8m envisaged. The adoption of precast driven piles should also limit the amount of arisings to be disposed of.

An alternative approach adopting deep trench fill below strip footings and pad foundations is also possible. However, the presence of made ground could result in significant costs in relation to the disposal of excavated soil from the site.

9. SUMMARY

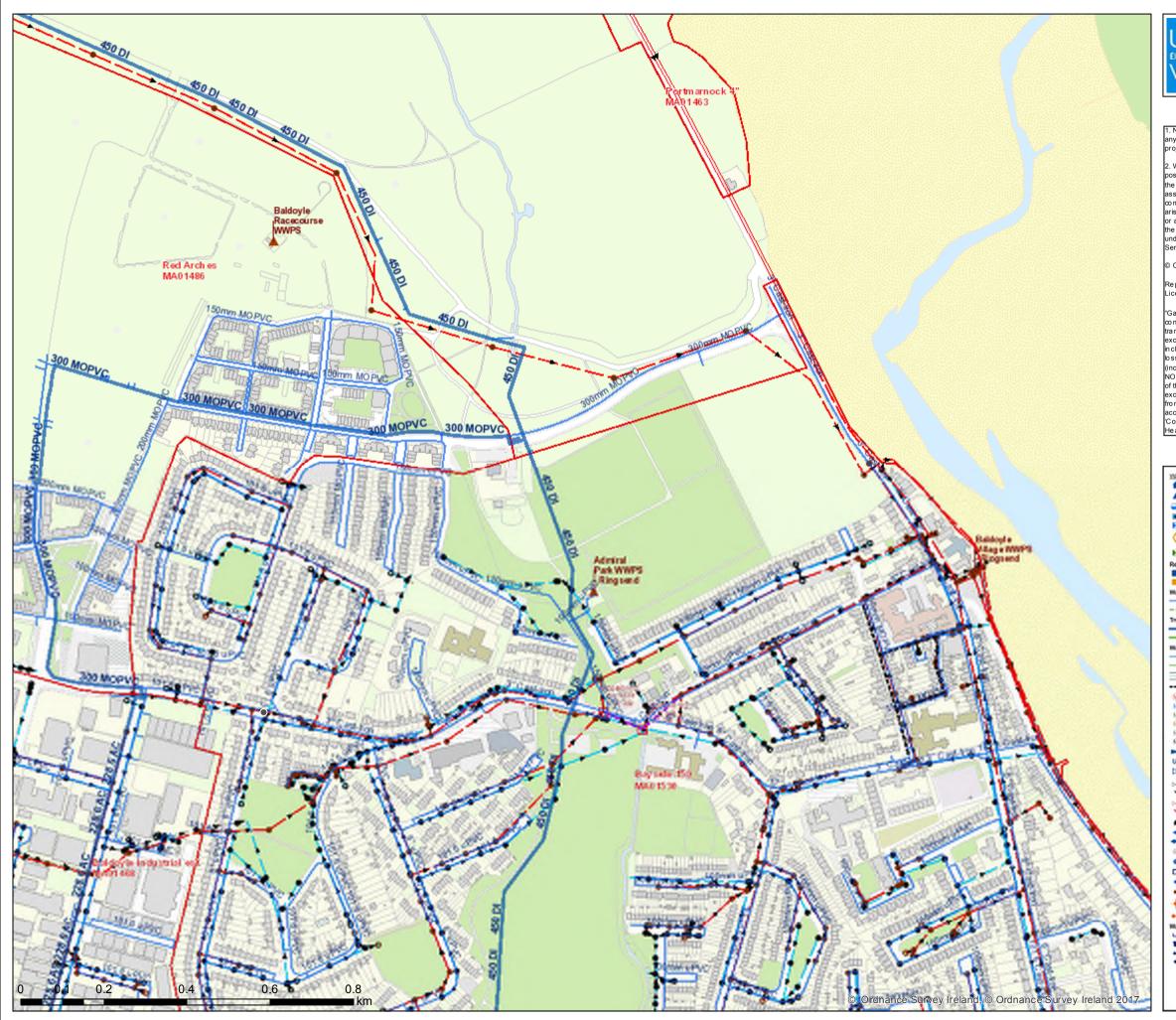
- The proposed development will have separate foul and surface water drainage system to serve the site.
- Surface water will be attenuated on site for the 1-in-100 year event including an allowance of +20% for climate change, and will discharge through a series of green roofs, an attenuation tank, permeable paving and an oversized pipe. The surface water system has been modelled in accordance with the Greater Dublin Area Strategic Drainage Study, with self-cleansing and discharge capacity is achieved based.
- A portion of the building will incorporate a green roof as a form of water treatment and to help slow the rate of surface water discharge from the site.
- Water will be supplied for this development via an existing connection located to the north of the site. The watermain has been designed fully in accordance with the Irish Water Code of Practice.
- The existing surface water outfall to FCC's infrastructure in Red Arches Road will be maintained.
- The existing foul drainage outfall to the west of the site will be maintained. New drainage elements within the site will be designed fully in accordance with the Irish Water Code of Practice.
- Flooding from external and or internal sources is not considered likely due to analysis of flood records for the area. Flood maps indicate that the site is within Flood Zone C (low risk >0.1% AEP).

APPENDIX A EXISTING SITE LAYOUT



APPENDIX B DRAINAGE AND WATERMAIN RECORDS

SR272-2021 Baldoyle





Print Date: 17/04/2021

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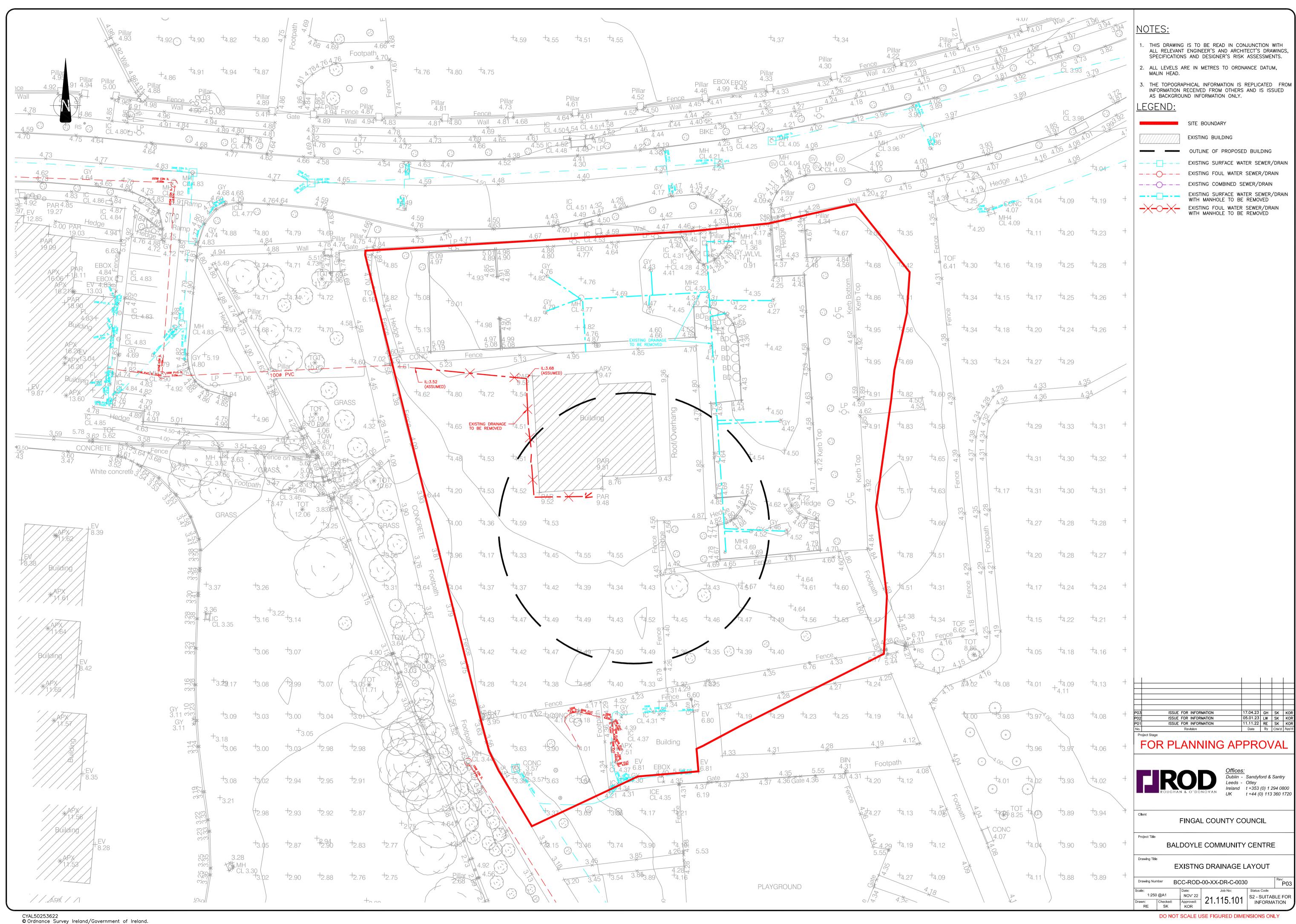
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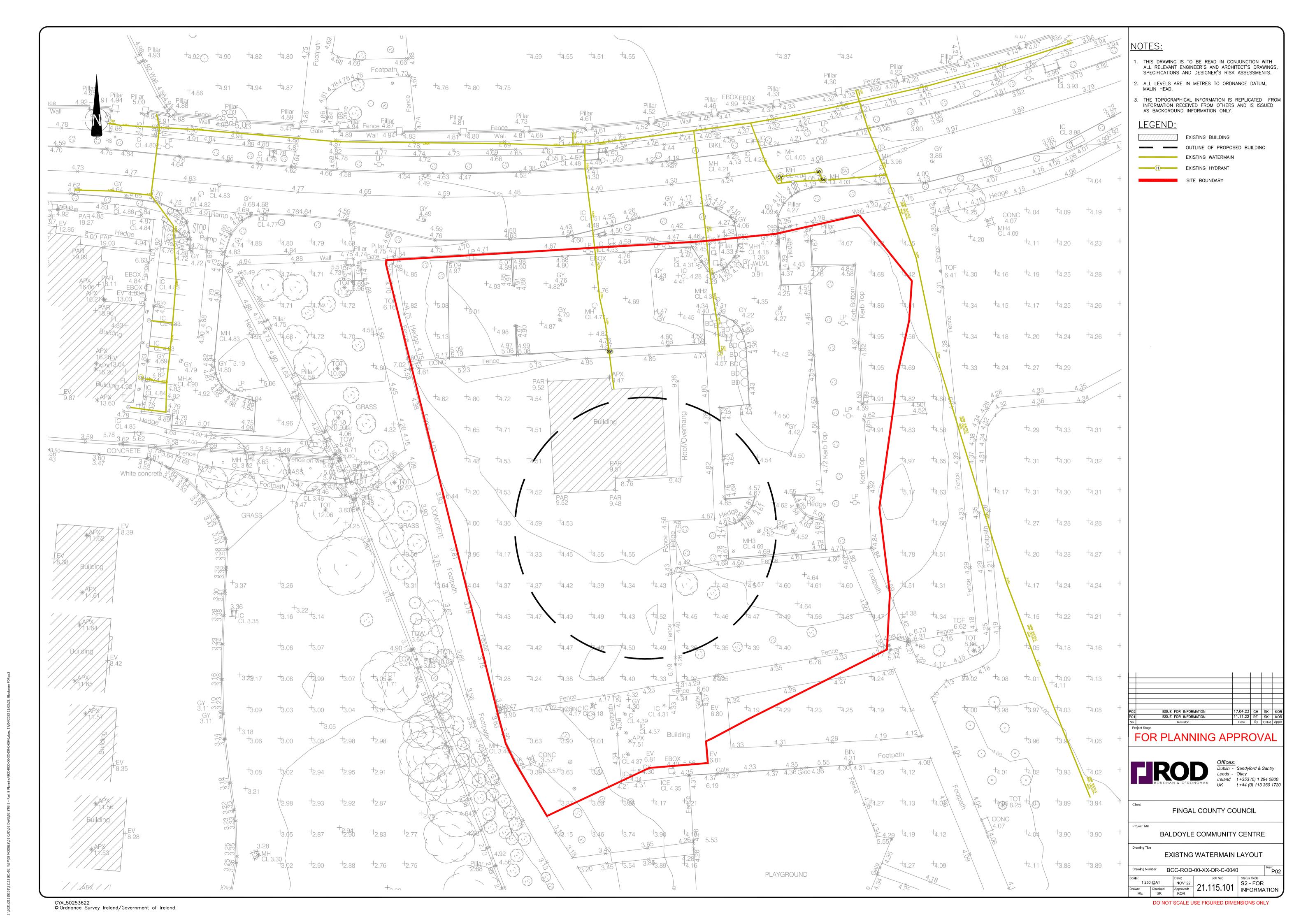
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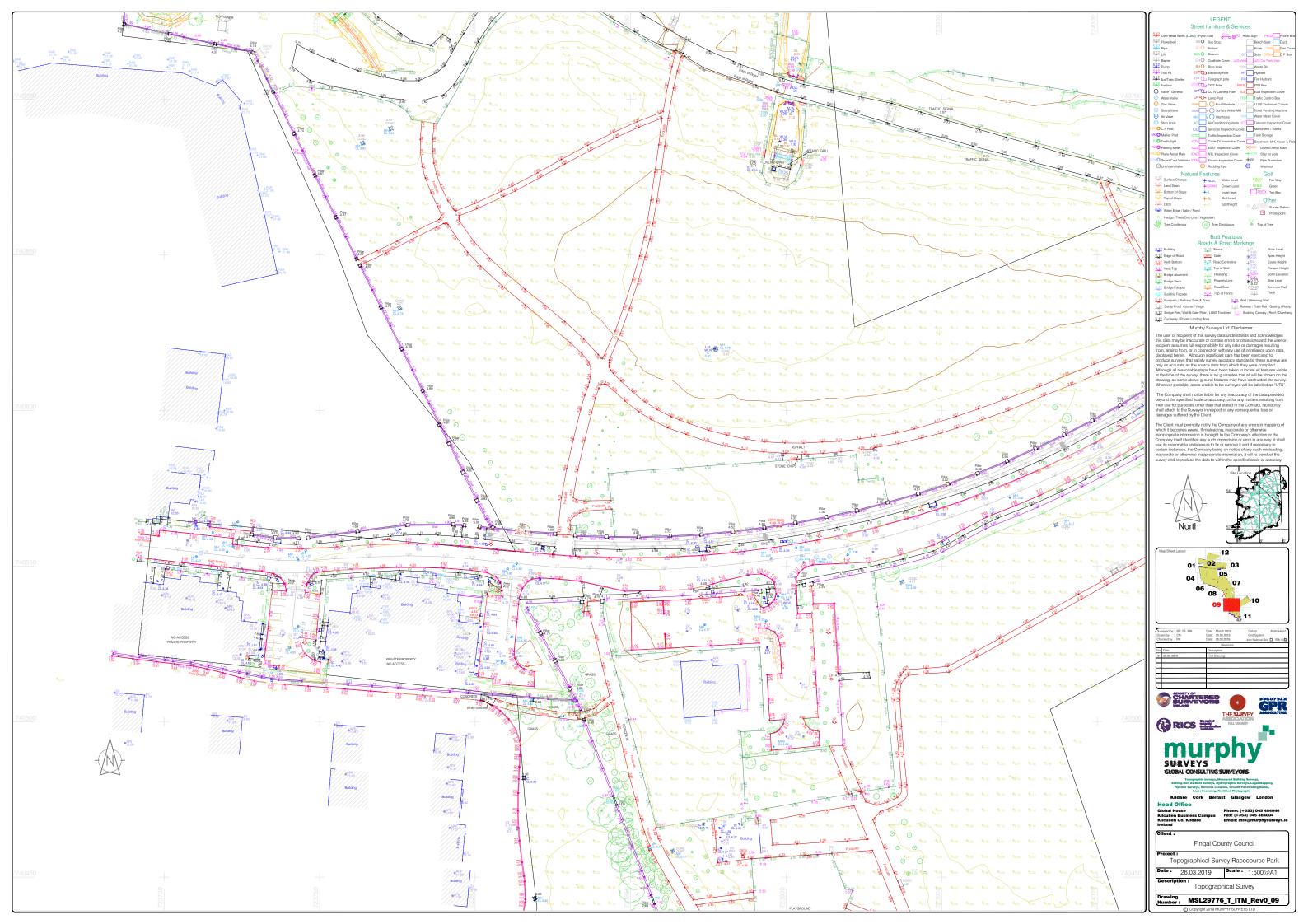
Code of Practice For Avoiding Danger From Underground Services' which is a vailable from the Health and Safety Authority (1890 28 93 89) or can be downloaded free of charge at www.hsa.ie."

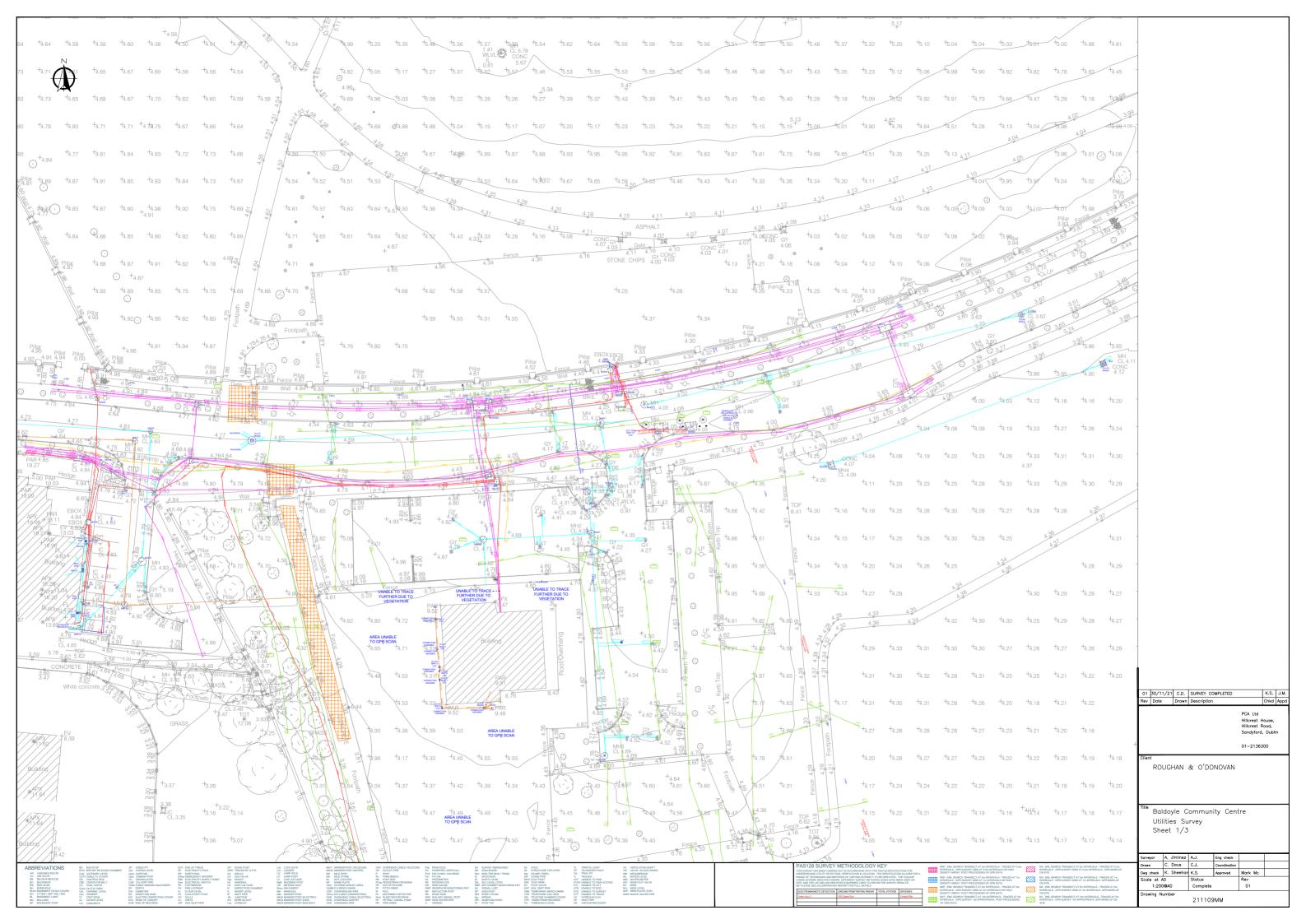






APPENDIX C TOPOGRAPHICAL & UTILITY SURVEY





APPENDIX D IRISH WATER CONNECTION OF FEASIBILITY



Kieran O'Riordan

Arena House, Arena Road, Sandyford Dublin 18 Dublin Uisce Éireann Bosca OP 448 Oifig Sheachadta na Cathrach Theas Cathair Chorcaí

1 April 2022

Irish Water PO Box 448, South City Delivery Office, Cork City.

www.water.ie

Re: CDS22002354 pre-connection enquiry - Subject to contract | Contract denied Connection for Business Connection of 1 unit(s) at Baldoyle Racecourse Park, Red Arches Road, Fingal

Dear Sir/Madam,

Irish Water has reviewed your pre-connection enquiry in relation to a Water & Wastewater connection at Baldoyle Racecourse Park, Red Arches Road, Fingal (the **Premises**). Based upon the details you have provided with your pre-connection enquiry and on our desk top analysis of the capacity currently available in the Irish Water network(s) as assessed by Irish Water, we wish to advise you that your proposed connection to the Irish Water network(s) can be facilitated at this moment in time.

SERVICE	OUTCOME OF PRE-CONNECTION ENQUIRY THIS IS NOT A CONNECTION OFFER. YOU MUST APPLY FOR A CONNECTION(S) TO THE IRISH WATER NETWORK(S) IF YOU WISH TO PROCEED.				
Water Carrier					
Water Connection	Feasible without infrastructure upgrade by Irish Water				
Wastewater Connection	Feasible without infrastructure upgrade by Irish Water				
	SITE SPECIFIC COMMENTS				
Water Connection					
Wastewater Connection	Separate storm and foul water connection services have to be provided for the Development. The surface and storm water from the site must be discharged only into an existing storm water network that does not discharge to an IW combined/foul sewer. The connection arrangement should be agreed with the Local Authority Drainage Division. The customer is responsible for obtaining all necessary consents/permissions required to facilitate any connection works to private infrastructure. The status and capacity of the infrastructure should be verified, prior to any physical connection works.				

The proposed development appears to connect to the Irish Water Network via private land/s. Please be advised that at connection application stage, you have to provide evidence of consent of the Third Party Landowner/s for the connection works to be carried out through these lands.

The design and construction of the Water & Wastewater pipes and related infrastructure to be installed in this development shall comply with the Irish Water Connections and Developer Services Standard Details and Codes of Practice that are available on the Irish Water website. Irish Water reserves the right to supplement these requirements with Codes of Practice and these will be issued with the connection agreement.

The map included below outlines the current Irish Water infrastructure adjacent to your site:



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General Notes:

- The initial assessment referred to above is carried out taking into account water demand and wastewater discharge volumes and infrastructure details on the date of the assessment. The availability of capacity may change at any date after this assessment.
- 2) This feedback does not constitute a contract in whole or in part to provide a connection to any Irish Water infrastructure. All feasibility assessments are subject to the constraints of the Irish Water Capital Investment Plan.
- 3) The feedback provided is subject to a Connection Agreement/contract being signed at a later date.
- 4) A Connection Agreement will be required to commencing the connection works associated with the enquiry this can be applied for at https://www.water.ie/connections/get-connected/
- 5) A Connection Agreement cannot be issued until all statutory approvals are successfully in place.
- 6) Irish Water Connection Policy/ Charges can be found at https://www.water.ie/connections/information/connection-charges/
- 7) Please note the Confirmation of Feasibility does not extend to your fire flow requirements.
- 8) Irish Water is not responsible for the management or disposal of storm water or ground waters. You are advised to contact the relevant Local Authority to discuss the management or disposal of proposed storm water or ground water discharges
- 9) To access Irish Water Maps email datarequests@water.ie
- 10) All works to the Irish Water infrastructure, including works in the Public Space, shall have to be carried out by Irish Water.

If you have any further questions, please contact Kevin McManmon from the design team at kmcmanmon@water.ie For further information, visit www.water.ie/connections.

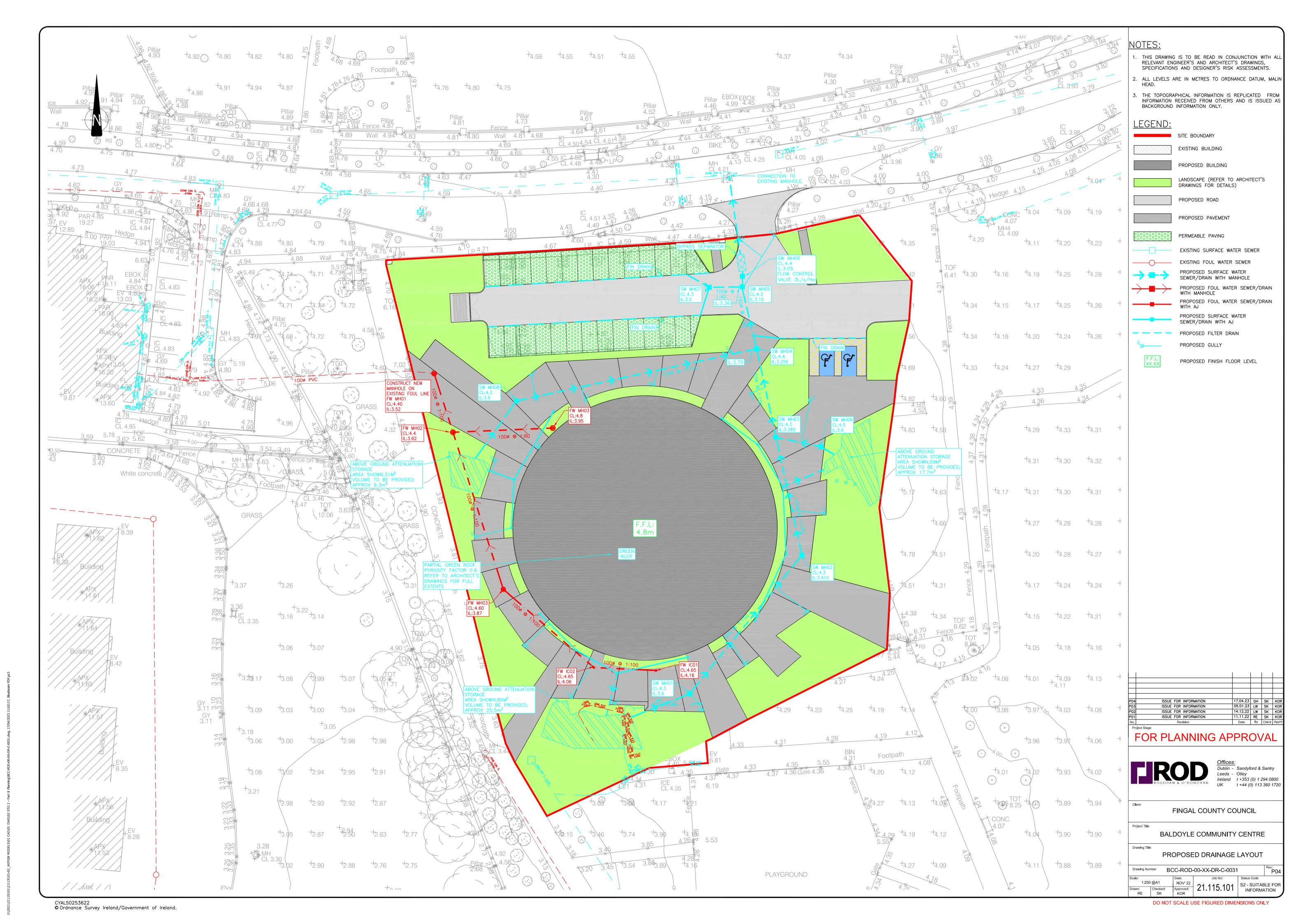
Yours sincerely,

Muonne Hallis

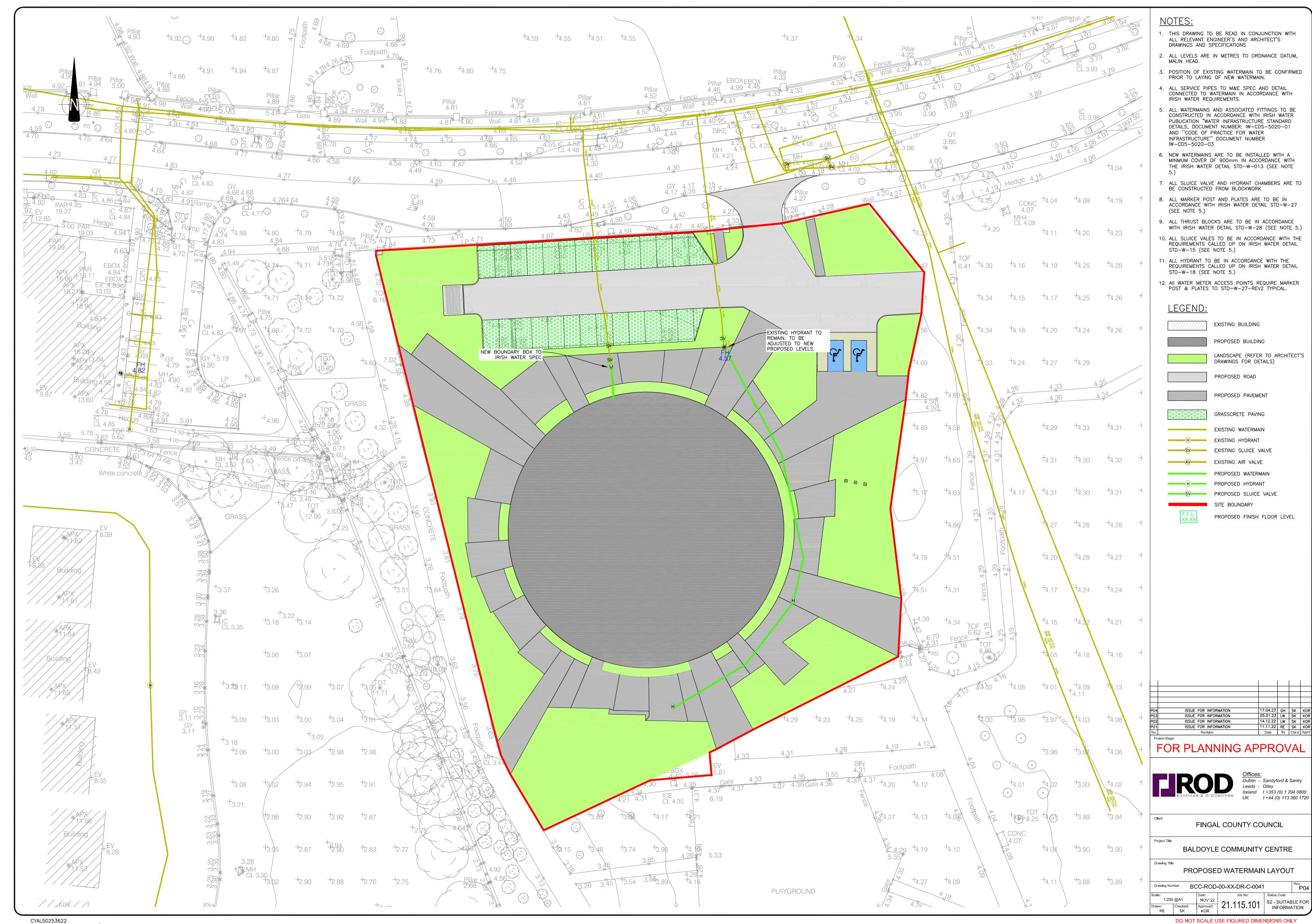
Yvonne Harris

Head of Customer Operations

APPENDIX E PROPOSED DRAINAGE LAYOUT



APPENDIX F PROPOSED WATERMAIN LAYOUT



Ordnance Survey Ireland/Government of Ireland.

APPENDIX G SURFACE WATER CALCULATIONS

Roughan & O'Donovan	Roughan & O'Donovan						
Arena House							
Arena Road							
Sandyford Dublin 18		Micro					
Date 31/01/2022 12:49	Designed by Sean. Kennedy	Drainage					
File 21.115.101 Baldoyle MD_SK 22.01.26.MDX	Checked by	Dialilade					
Micro Drainage	Network 2019.1						

Manhole Schedules for Storm

MH Name	MH CL (m)	MH Depth (m)	Coni	MH nection	MH Diam.,L*W (mm)	PN	Pipe Out Invert Level (m)	Diameter (mm)	PN	Pipes In Invert Level (m)	Diameter (mm)	Backdrop (mm)
IC01	4.375	0.725	Open	Manhole	1200	1.000	3.650	100				
01	4.300	0.700	Open	Manhole	1200	1.001	3.600	150	1.000	3.600	100	
02	4.300	0.890	Open	Manhole	1200	1.002	3.410	150	1.001	3.410	150	
4	4.300	1.018	Open	Manhole	1200	2.000	3.282	150				
09	4.500	1.244	Open	Manhole	1200	2.001	3.600	100	2.000	3.256	150	
03	4.300	1.018	Open	Manhole	1200	1.003	3.282	100	1.002	3.282	150	
									2.001	3.282	100	
IC02	4.375	0.725	Open	Manhole	1200	3.000	3.650	100				
08	4.300	0.700	Open	Manhole	1200	3.001	3.600	100	3.000	3.600	100	
04	4.400	1.144	Open	Manhole	1200	1.004	3.256	150	1.003	3.256	100	
									3.001	3.256	100	
07	4.300	0.800	Open	Manhole	1200	4.000	3.500	150				
05	4.300	1.200	Open	Manhole	1200	1.005	3.100	150	1.004	3.100	150	
									4.000	3.100	150	
06	4.400	1.350	Open	Manhole	1200	1.006	3.050	100	1.005	3.050	150	
	4.300	1.531	Open	Manhole	0		OUTFALL		1.006	2.769	100	

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Roughan & O'Donovan				
Arena House				
Arena Road				
Sandyford Dublin 18		Micro		
Date 31/01/2022 12:49	Designed by Sean.Kennedy	Drainage		
File 21.115.101 Baldoyle MD_SK 22.01.26.MDX	Checked by	Drainage		
Micro Drainage	Network 2019.1			

Manhole Schedules for Storm

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
IC01	723878.401	740476.785	723878.401	740476.785	Required	
01	723884.772	740471.556	723884.772	740471.556	Required	
02	723905.148	740491.511	723905.148	740491.511	Required	٨
4	723910.172	740504.791	723910.172	740504.791	Required	
09	723910.056	740508.727	723910.056	740508.727	Required	•
03	723902.083	740510.445	723902.083	740510.445	Required	ļ
IC02	723861.391	740507.860	723861.391	740507.860	Required	
08	723861.150	740516.275	723861.150	740516.275	Required	•
						!

Roughan & O'Donovan				
Arena House				
Arena Road				
Sandyford Dublin 18		Micro		
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File 21.115.101 Baldoyle MD_SK 22.01.26.MDX	Checked by	Drainage		
Micro Drainage	Network 2019.1			

Manhole Schedules for Storm

MH Name	Manhole Easting (m)	Manhole Northing (m)	Intersection Easting (m)	Intersection Northing (m)	Manhole Access	Layout (North)
04	723899.739	740524.510	723899.739	740524.510	Required	
07	723892.131	740534.386	723892.131	740534.386	Required	•—
05	723897.455	740534.427	723897.455	740534.427	Required	
06	723897.575	740538.575	723897.575	740538.575	Required	Ĭ,
	723895.291	740552.459			No Entry	•

Roughan & O'Donovan		Page 4
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Arena Road		
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Micro Drainage	Network 2019.1	

PIPELINE SCHEDULES for Storm

<u>Upstream Manhole</u>

PN	Hyd Sect		MH Name	C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	0	100	IC01	4.375	3.650	0.625	Open Manhole	1200
1.001	0	150	01	4.300	3.600	0.550	Open Manhole	1200
1.002	0	150	02	4.300	3.410	0.740	Open Manhole	1200
2.000	0	150	4	4.300	3.282	0 868	Open Manhole	1200
2.001	0	100	0.9	4.500	3.600		Open Manhole	1200
2.001	O	100	09	4.500	3.000	0.000	Open Mannore	1200
1.003	0	100	03	4.300	3.282	0.918	Open Manhole	1200

Downstream Manhole

PN	Length (m)	Slope (1:X)		C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
1.000	8.242	164.8	01	4.300	3.600	0.600	Open Manhole	1200
1.001	28.520	150.1	02	4.300	3.410	0.740	Open Manhole	1200
1.002	19.180	149.8	03	4.300	3.282	0.868	Open Manhole	1200
2.000	3.938	150.0	09	4.500	3.256	1.094	Open Manhole	1200
2.001	8.156	25.6	03	4.300	3.282	0.918	Open Manhole	1200
1.003	14.259	548.4	04	4.400	3.256	1.044	Open Manhole	1200

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Roughan & O'Donovan		Page 5
Arena House		
Arena Road		
Sandyford Dublin 18		Micro
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Micro Drainage	Network 2019.1	

PIPELINE SCHEDULES for Storm

<u>Upstream Manhole</u>

PN	-	Diam (mm)		C.Level (m)	I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*W (mm)
3.000 3.001	0	7.00	IC02 08	4.375 4.300	3.650 3.600		Open Manhole Open Manhole	1200 1200
1.004	0	150	04	4.400	3.256	0.994	Open Manhole	1200
4.000	0	150	07	4.300	3.500	0.650	Open Manhole	1200
1.005 1.006	0	150 100	05 06	4.300 4.400	3.100 3.050		Open Manhole Open Manhole	1200 1200

Downstream Manhole

PN	Length (m)	Slope (1:X)			I.Level (m)	D.Depth (m)	MH Connection	MH DIAM., L*
	8.418		08	4.300			Open Manhole	120
3.001	39.457	114.7	04	4.400	3.256	1.044	Open Manhole	120
1.004	10.177	65.2	05	4.300	3.100	1.050	Open Manhole	120
4.000	5.323	13.3	05	4.300	3.100	1.050	Open Manhole	120
1.005	4.149	83.0	06	4.400	3.050	1.200	Open Manhole	120
1.006	14.071	50.0		4.300	2.769	1.431	Open Manhole	
				©1982	-2019 I	nnovyze	9	

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Arena Road		
Sandyford Dublin 18		Micro
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Micro Drainage	Network 2019.1	

Free Flowing Outfall Details for Storm

Outfall Outfall C. Level I. Level Min D,L W
Pipe Number Name (m) (m) I. Level (mm) (mm)
(m)

1.006 4.300 2.769 0.000 0

Simulation Criteria for Storm

Volumetric Runoff Coeff 0.750 Manhole Headloss Coeff (Global) 0.500 Inlet Coefficient 0.800

Areal Reduction Factor 1.000 Foul Sewage per hectare (1/s) 0.000 Flow per Person per Day (1/per/day) 0.000

Hot Start (mins) 0 Additional Flow - % of Total Flow 0.000 Run Time (mins) 60

Hot Start Level (mm) 0 MADD Factor * 10m³/ha Storage 2.000 Output Interval (mins) 1

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 4 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 16.200 Cv (Summer) 0.750 Return Period (years) 100 Ratio R 0.300 Cv (Winter) 0.840 Region Scotland and Ireland Profile Type Summer Storm Duration (mins) 30

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Micro Drainage	Network 2019.1	

Online Controls for Storm

Hydro-Brake® Optimum Manhole: 06, DS/PN: 1.006, Volume (m³): 1.6

Unit Reference	MD-SHE-0069-2000-0900-2000	Sum	p Available	Yes
Design Head (m)	0.900	Di	ameter (mm)	69
Design Flow $(1/s)$	2.0	Inver	t Level (m)	3.050
Flush-Flo™	Calculated	Minimum Outlet Pipe Di	ameter (mm)	100
Objective	Minimise upstream storage	Suggested Manhole Di	ameter (mm)	1200
Application	Surface			

Control	Points	Head (m)	Flow (1/s)	Control Points	Head (m)	Flow (1/s)
Design Point	(Calculated)	0.900	2.0	Kick-Flo®	0.568	1.6
	Flush-Flo™	0.278	2.0	Mean Flow over Head Range	_	1.8

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (1/s)										
0.100	1.7	0.600	1.7	1.600	2.6	2.600	3.2	5.000	4.4	7.500	5.3
0.200	2.0	0.800	1.9	1.800	2.7	3.000	3.5	5.500	4.6	8.000	5.5
0.300	2.0	1.000	2.1	2.000	2.9	3.500	3.7	6.000	4.8	8.500	5.7
0.400	1.9	1.200	2.3	2.200	3.0	4.000	4.0	6.500	5.0	9.000	5.8
0.500	1.8	1.400	2.4	2.400	3.1	4.500	4.2	7.000	5.2	9.500	6.0

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Arena Road		
Sandyford Dublin 18		Micro
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Micro Drainage	Network 2019.1	

Storage Structures for Storm

Tank or Pond Manhole: ICO1, DS/PN: 1.000

Invert Level (m) 3.650

Depth (m) Area (m²) Depth (m) Area (m²)

0.000 35.0 0.300 35.0

Tank or Pond Manhole: 4, DS/PN: 2.000

Invert Level (m) 3.282

Depth (m) Area (m²) Depth (m) Area (m²)

0.000 234.0 0.250 234.0

Tank or Pond Manhole: ICO2, DS/PN: 3.000

Invert Level (m) 3.650

Depth (m) Area (m²) Depth (m) Area (m²)

0.000 45.0 0.300 45.0

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Micro Drainage	Network 2019.1	1

Porous Car Park Manhole: 07, DS/PN: 4.000

0.0	Slope (1:X)		0.30	sity	Poros	0.00000	Infiltration Coefficient Base (m/hr)
5	Storage (mm)	Depression	3.500	(m)	Invert Level	1000	Membrane Percolation (mm/hr)
3	tion (mm/day)	Evaporat	10.0	(m)	Width	111.1	Max Percolation (1/s)
0	ne Depth (mm)	Membra	40.0	(m)	Length	2.0	Safety Factor

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Micro Drainage	Network 2019.1	

Simulation Criteria

Areal Reduction Factor 1.000 Manhole Headloss Coeff (Global) 0.500 MADD Factor * 10m³/ha Storage 2.000
Hot Start (mins) 0 Foul Sewage per hectare (1/s) 0.000 Inlet Coefficient 0.800
Hot Start Level (mm) 0 Additional Flow - % of Total Flow 0.000 Flow per Person per Day (1/per/day) 0.000

Number of Input Hydrographs 0 Number of Offline Controls 0 Number of Time/Area Diagrams 0 Number of Online Controls 1 Number of Storage Structures 4 Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR M5-60 (mm) 16.200 Cv (Summer) 0.750 Region Scotland and Ireland Ratio R 0.300 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0 DVD Status ON Analysis Timestep 2.5 Second Increment (Extended) Inertia Status ON DTS Status

Profile(s)

Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080

Return Period(s) (years)

Climate Change (%)

Summer and Winter 4320, 5760, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080

									Water	Surcharged	${\tt Flooded}$			Pipe	
	US/MH		Return	Climate	First (X)	First (Y)	First (Z)	Overflow	Level	Depth	Volume	Flow /	Overflow	Flow	
PN	Name	Storm	Period	Change	Surcharge	Flood	Overflow	Act.	(m)	(m)	(m³)	Cap.	(1/s)	(l/s)	Status
1.000	IC01	60 Winter	100	+20%					3.746	-0.004	0.000	0.59		2.5	OK
						©1	982-2019	Innovyze	<u> </u>						

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Arena Road			
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US/MH Level
PN Name Exceeded

1.000 IC01

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Micro Drainage	Network 2019.1	

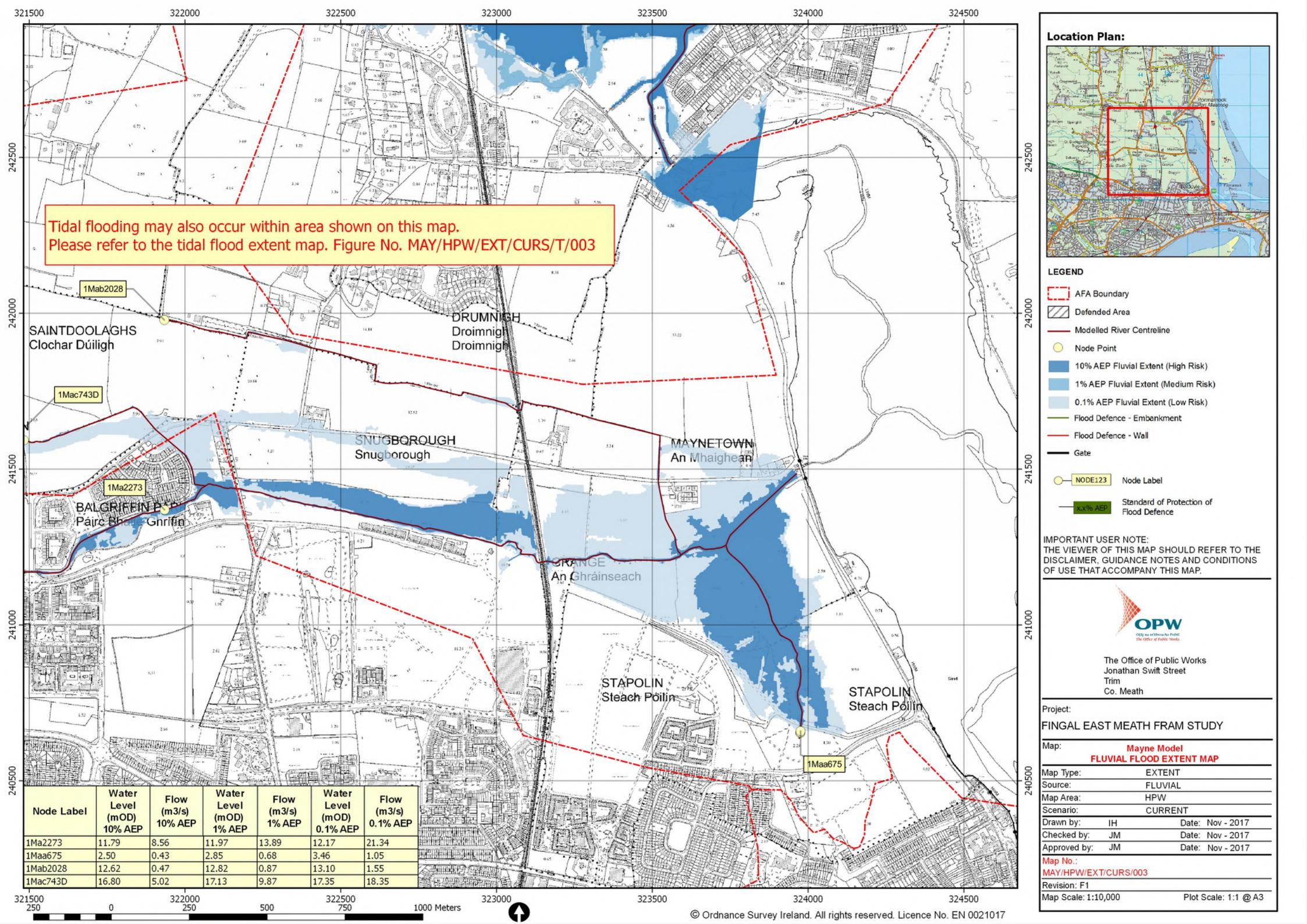
PN	US/MH Name	Storm		Climate Change	First (X) Surcharge	First (Y)	First (Z) Overflow	Overflow Act.		Surcharged Depth (m)	Flooded Volume (m³)	Flow /	Overflow (1/s)	Pipe Flow (1/s)
									\ /	, ,	\ <i>,</i>		(-, -,	(-/-/
1.001	01	60 Winter	100	+20%					3.728	-0.022	0.000	0.20		2.7
1.002	02	60 Winter	100	+20%	30/15 Summer				3.712	0.152	0.000	0.20		2.7
2.000	4	10080 Winter	100	+20%	30/2880 Winter				3.484	0.052	0.000	0.05		0.6
2.001	09	10080 Winter	100	+20%					3.501	-0.199	0.000	0.00		0.0
1.003	03	60 Winter	100	+20%	1/120 Winter				3.699	0.317	0.000	0.91		2.2
3.000	IC02	60 Winter	100	+20%	100/60 Winter				3.753	0.003	0.000	0.54		2.3
3.001	08	60 Winter	100	+20%	30/30 Winter				3.736	0.036	0.000	0.41		2.3
1.004	04	180 Winter	100	+20%	30/15 Summer				3.690	0.284	0.000	0.22		4.3
4.000	07	180 Winter	100	+20%	100/120 Summer				3.682	0.032	0.000	0.05		2.0
1.005	05	180 Winter	100	+20%	1/60 Winter				3.686	0.436	0.000	0.17		2.4
1.006	06	360 Winter	100	+20%	1/15 Summer				3.700	0.550	0.000	0.24		1.9

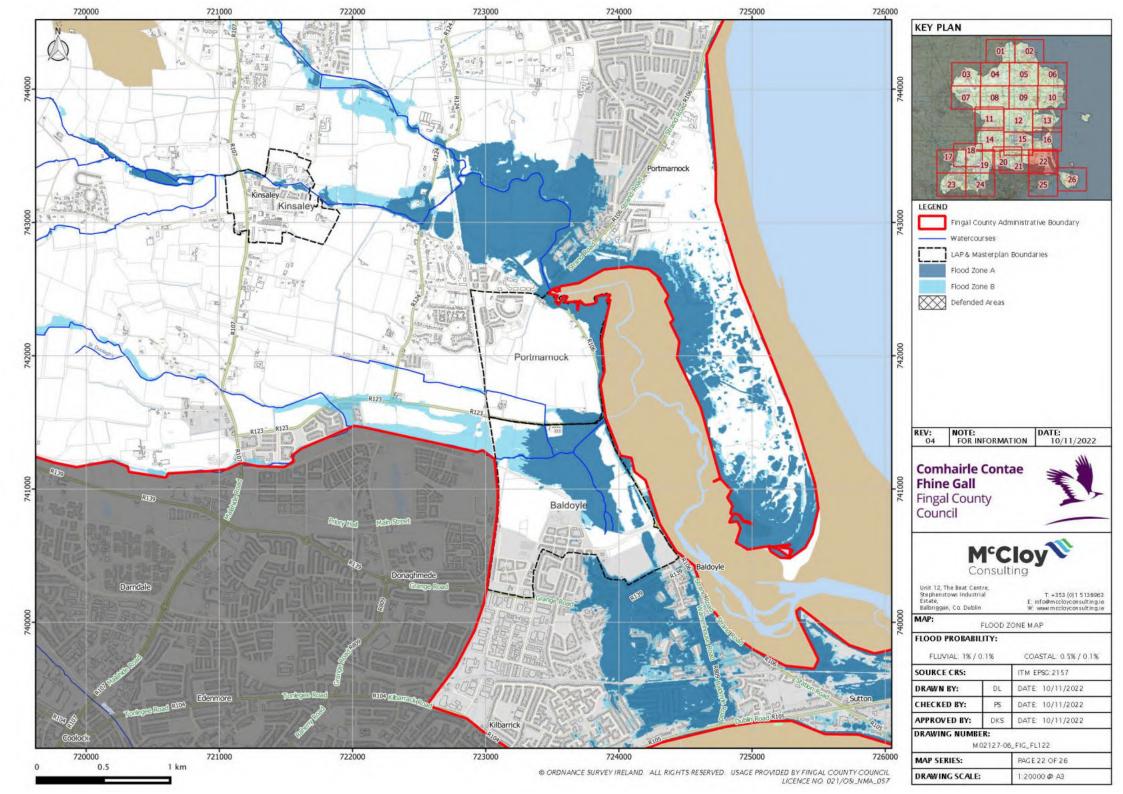
	US/MH		Level
PN	Name	Status	Exceeded
1.001	01	OK	
1.002	02	SURCHARGED	
2.000	4	SURCHARGED	
2.001	09	OK	
1.003	03	SURCHARGED	
3.000	IC02	SURCHARGED	
3.001	08	SURCHARGED	
1.004	04	SURCHARGED	
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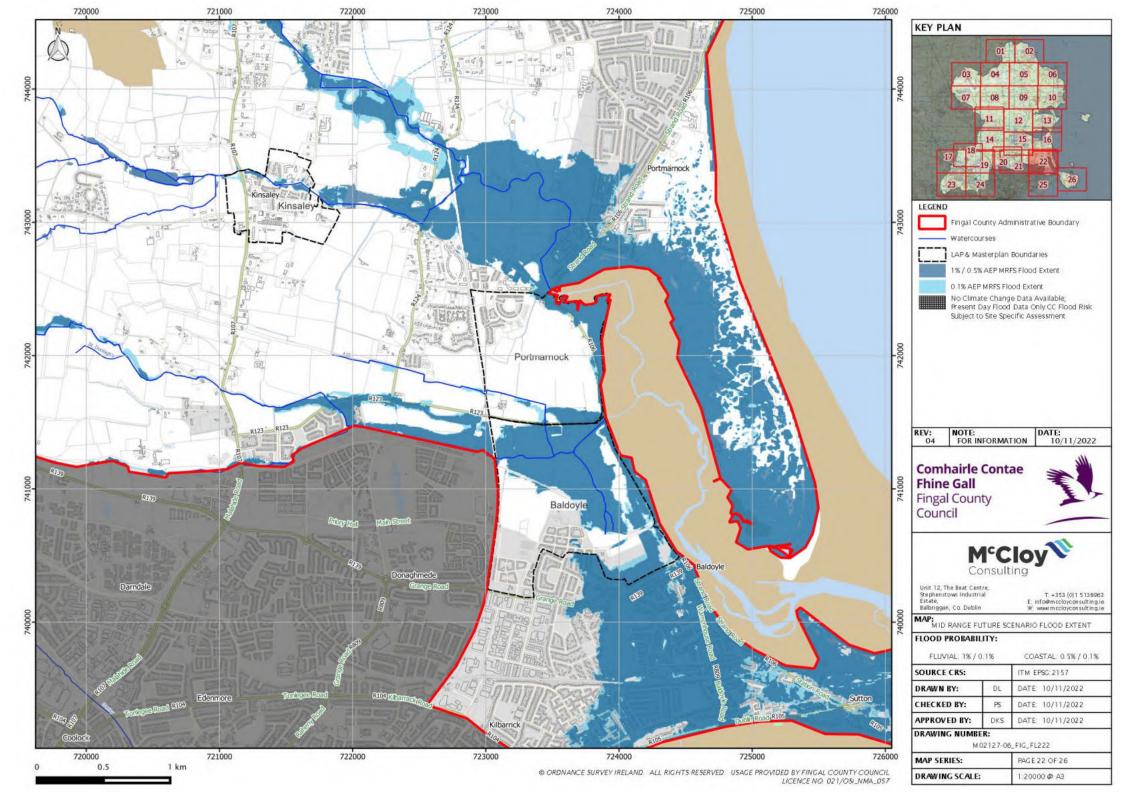
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File 21.115.101 Baldoyle MD_SK 22.01.26.MDX	Checked by	Drail lage
Micro Drainage	Network 2019.1	•

	US/MH		Level
PN	Name	Status	Exceeded
4.000	07	SURCHARGED	
1.005	05	SURCHARGED	
1.006	06	SURCHARGED	

APPENDIX H OPW & MCCLOY CONSULTING FLOOD RISK MAPPING







APPENDIX I FINGAL COUNTY COUNCIL TAKING-IN-CHARGE DRAWINGS

