# Harry Reynolds Road Pedestrian and Cycle Route 

Feasibility Study and Options Assessment Report

Fingal County Council

October 2018

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This document has 79 pages including the cover.
Revision history

| Rev | Date | Description | Originator | Checker | Approver |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 01 | $03 / 10 / 18$ | Issued to FCC | BH | SW | MF |
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Introduction

## 1. Introduction

### 1.1. Background

Fingal County Council proposes to develop a new pedestrian and cycle facility along the Harry Reynolds Road, Balbriggan, Co Dublin. The proposed scheme will aim to deliver a minimum Level of Service A in accordance with the National Cycle Manual and will allow for future possible links to a coastal greenway and other cycling routes in Balbriggan.

To achieve this objective, Atkins have been engaged by Fingal County Council for all stages relevant to the delivery of the project including to identify and assess concept route options at this stage.

### 1.2. Aims and Objectives

The main aims and objectives of this Feasibility Study Options Report are listed below:

- To consider the context of the scheme in terms of Local and Regional Planning Policy.
- To identify significant engineering and environmental constraints.
- To set out the route options considered and to summarise their feasibility and relative ranking in terms of various relevant criteria.
- To appraise the route options and make a recommendation in relation to a preferred concept route option.


### 1.3. Methodology

The following items have been undertaken in order to complete this Feasibility Study and Options Assessment Report:

- A desktop study was carried out including a review of regional and local planning policy information, a review of engineering constraints and a review of environmental constraints.
- Topographical data, utility information and traffic information were collected.
- Site inspections were carried out to ensure information was up to date and correct.
- All known significant constraints were collated and mapped.
- Route options were developed having due regard to the identified constraints.
- Route options were appraised in a comparative manner, resulting in the recommendation of a preferred route.


### 1.4. Study Area

The study area upon which this Feasibility Study and Options Assessment is based is divided into two distinct sections. The extents of both study areas are shown in Figure 1-1 below.

Section One encompasses the extents of Harry Reynolds Road from its beginning at the Drogheda Street junction to the north of Balbriggan town centre extending westward linking with Chieftain's Drive and Moylaragh Road. The remainder of Harry Reynolds Road extends in a generally southward direction where it terminates at the Dublin Street roundabout junction. This section then extends further east along Hamilton Road towards Castleland.

Section Two encompasses the extents of the parklands surrounding the Bracken River. Located to the east of Dublin Street.


Figure 1-1 Study area

## 2. Scheme Context

### 2.1. Planning Policy

A number of Regional and Local planning policies have been considered as part of this study and are discussed below.

### 2.1.1. Greater Dublin Area Cycle Network Plan

The Greater Dublin Area Cycle Network Plan was published by the NTA in 2013 and sets out the proposed cycle network in the Greater Dublin Area. The GDA categorises the proposed route along Harry Reynolds Road as BA2, a Primary/Secondary route. Figure 2-1 below is an extract from the GDA showing the categorisation of proposed routes in the Balbriggan area.


Figure 2-1 GDA route categorization in the Balbriggan area
There is also a feeder link within the park surrounding the Bracken River. An additional feeder link is also noted adjacent to the cemetery on Chapel Street. Both feeder links connect to the BA2 Primary/Secondary Route and are within the Study Area.

### 2.1.2. County Policy

The Fingal County Development Plan 2018-2023 sets out to promote and facilitate movement within and to the County through the integration of land use with a sustainable transport system, with priority given to public transport, walking and cycling. There are several relevant chapters in the Development Plan which relate to Cycling and Walking. The main objectives relevant to this scheme are summarised below:

- Objective 11: Ensure a safe and convenient road, pedestrian and cycle system promoting permeability, accessibility and connectivity between existing and new developments within the town. (Chapter 4).
- Objective MT17: Improve pedestrian and cycle connectivity to schools and third level colleges and identify and minimise barriers to children walking and cycling to primary and secondary schools. Parks, Open Space Recreation theme. (Chapter 7).
- Objective G126: Maximise the use and potential of existing parks, open space and recreational provision, both passive and active, by integrating existing facilities where appropriate. (Chapter 8).
- Objective G127: Provide a range of accessible new parks, open space and recreational facilities accommodating a wide variety of uses (both passive and active), use intensities and interests. (Chapter 8).
- Objective G128: Provide attractive and safe routes linking key green space sites, parks and open spaces and other foci such as cultural sites and heritage assets as an integral part of a new green infrastructure provision, where appropriate and feasible. (Chapter 8).

Figure 2-2 below displays the planning objectives for the area surrounding the proposed pedestrian and cyclist facility.


Figure 2-2
Fingal Development Plan 2017-2023 planning objectives
There are two masterplan areas in the vicinity of the proposed pedestrian and cycle facility, both of which lie in the southern section of the scheme.

- MP 4.C - Millpond Masterplan

Facilitate the development of Mill Pond to provide for passive and active recreational facilities and amenities including a feasibility study to develop the lake for the purposes of wildlife promotion.

- MP 4.D - Stephenstown Masterplan

Provide for architecturally designed buildings with high quality finishes fronting onto the Naul Road. The development of lands in this area will be guided by the principles contained in the 'Stephenstown Urban Design and Landscape Masterplan (2009).

The development plan also highlights the proposed cycle and pedestrian routes. These routes can be viewed in Figure 2-2 which are denoted by the symbol shown below. With the exception of Moylaragh Road, all of the route has a cycle/pedestrian route objective.


There is also a provision for a new road link to the Skerries road at the eastern end of Hamilton road. This link will allow traffic from the Skerries area with a route that avoids the town centre and should reduce and redistribute traffic from Dublin Street to Hamilton Road. .

## 3. Background

### 3.1. Land Use, Planning and Land Ownership

Land use varies throughout the extents of the scheme. Figure 3-1 below depicts the typical land uses surrounding the proposed facility.

There are a large number of residential developments in the vicinity of the proposed facility. In the northern extremities the proposed route is surrounded by residential developments. As the scheme proceeds in a southeast direction the residential developments continue along the eastern side of Harry Reynolds Road, whilst on the west side there is a cemetery and undeveloped zoned lands. Finally, in the southern section of the scheme the major residential developments are located to the north along Hamilton Road.

A number of designated business and retail areas are located along and close to the route. Balbriggan town centre is located to the east of the scheme. Stephenstown Industrial Estate encompasses the southern section of Harry Reynolds Road. Millfield Shopping Centre, the major retail attraction in the area, is located to the east of the proposed route, along Chapel Street.

A number of education centres are noted in the vicinity of the scheme, a large number of which will be accessible from the proposed route. Figure 3-1 below shows the location of these education centres.


Figure 3-1 Land use

### 3.2. Population

Figure 3-2 shows the small area population map for Balbriggan (source: 2016 Census). With reference to this it is clear that the north side of the town has higher densities than the southern section.


Figure 3-2 SAPMAP for Balbriggan

The Figure 3-3 below shows the areas reachable in 5, 10 and 15 minutes cycle from the proposed route.


Figure 3-3 Harry Reynolds Road Cycling Times
The population statistics from the 2016 Census Small Areas for each of the above time periods is summarised in the table below.

Table 3-1 2016 Population Statistics

| Time to Cycle | Population |
| :---: | :---: |
| $0-5$ Mins | 17,904 |
| $5-10$ Mins | 21,106 |
| $10-15$ Mins | 22,756 |

From the above data it is clear that the provision of cycle facilities on Harry Reynolds Road will provide excellent service to the majority of those living within Balbriggan. Given its central location, the majority of the population is within 5 minutes cycling distance from the facilities.

### 3.3. Collision History

The RSA online accident database was reviewed to identify accidents within the Study Area. In total 17 no. accidents occurred from 2004 to 2014 along the proposed cycleway route. All of which were categorised as minor accidents. Two of these accidents involved pedestrians, one of which took place at the junction of Harry Reynolds Rd and Chapel Court. The second taking place at the roundabout junction at Harry Reynolds Rd and the R132. An incident also occurred involving a cyclist over the same timeline, this occurred at the junction of Harry Reynolds Rd and the R132. An overview of the collision history is shown in Figure 3-4 the figure below.


Figure 3-4 Collision History

### 3.4. Road Infrastructure Review

For the purpose of reviewing the existing road infrastructure, the route was broken up into eight sections as shown in Figure 3-5. Each section is discussed in detail in the following section and are generally described travelling from north to south.


Figure 3-5 Existing road infrastructure sections

### 3.4.1. Section 1

Section 1 extends from the junction between Harry Reynolds Road and Drogheda Street (R132) in a southwesterly direction to the roundabout junction where Harry Reynolds Road turns southward. The single carriageway has an overall width of approximately 6.5 m . Footpaths run adjacent to the carriageway on either side and a buffer is provided by means of a grass verge. There are a number of T-junctions with local estate roads along this section of the route. This section of the scheme has a 50 kph speed limit in place.

### 3.4.2. Section 2

Section 2 covers the link road between the roundabout in Section 1 and the roundabout at Moylaragh Road. The single lane carriageway has an overall width of approximately 8 m . Footpath and cycle lanes run adjacent to the carriageway on both sides. A buffer between the footpath and the carriageway is provided through a grass verge. There is one T-junction with a local estate road and a relatively long section of parallel parking for residents on the southern side of this section of the route. This section of the scheme has a 50 kph speed limit in place.

### 3.4.3. Section 3

Section 3 covers the Moylaragh Road between the roundabout in Section 2 leading up to the signal controlled junction at Castlemill Link. The single lane carriageway has an overall width of approximately $6.5-7.0 \mathrm{~m}$. A footpath runs adjacent to the carriageway on the southern side which is separated from the carriageway by means of a grass verge. Residential properties and driveways directly front along this section of road along the majority of its length with a number of T-junctions providing access to the rest of the estate. This section also includes the grassed park area within Moylaragh. This section of the scheme has a 50 kph speed limit in place.

### 3.4.4. Section 4

Section 4 extends along Harry Reynolds Road from the roundabout in Section 3 to the signal controlled junction with Chapel Street. The single carriageway has an overall width of approximately $9.0-10.0 \mathrm{~m}$. Footpaths run adjacent to the carriageway on either side. A buffer is provided by means of a grass verge also on both sides. A number of access junctions to residential streets are present along the carriageway. A cycle lane develops towards the south of the section on the eastern side. This section of the scheme has a 50 kph speed limit in place.

### 3.4.5. Sections 5 and 6

Section 5 and 6 covers the link road between Harry Reynolds Road and the three-arm roundabout to the south of Jack Murphy Outdoor Clothing. The single carriageway has an overall width of approximately 9.0 m . Footpaths run adjacent to the carriageway on either side. A number of access junctions to industrial and commercial units are present along the carriageway. A permitted speed limit of 60 kph is allocated along section 5 and section 6.

### 3.4.6. Section 7

Section 7 covers the exit road from the public car park opposite to St Peter and Pauls Church. The carriageway is approximately 6.0 m and is one-way road westwards only. There is an existing footpath to the north of the road with a grass buffer while there is grass verge and vegetation to the southern side. The footpath links to the existing roundabout on Dublin Street along the perimeter wall of the car park.

### 3.4.7. Section 8

Section 8 covers the link road between the roundabout on Dublin Street and the three arm roundabout on Castle Park Avenue. The single carriageway has an overall width of approximately 8.0 m . Footpaths and cycle lanes run adjacent to the carriageway on either side. A buffer is provide along both sides by means of a grass verge. A speed limit of 60 kph is in place along this section of the scheme.

### 3.4.8. Key Junctions

There are four key junctions within the extents of the scheme. Two of which are signal controlled junctions with the remaining two being a three and four arm roundabout. These are identified in Figure 3-5 above.

### 3.5. The Park

The existing park that runs parallel to Dublin Street at the southern end of Harry Reynolds Road is generally linear in nature and follows the Bracken River. There are existing footpaths through the park which link to Harry Reynolds Road, the public car park, Clonard Court and Vauxhall Street. There are two existing bridge crossings of the river which are approximately $1.8 m-2.0 m$ wide.

A Masterplan is currently being prepared for this parkland area and a proposed new skate park is to be constructed within the next year.

### 3.6. Utilities

Utilities companies and service providers were contacted to determine presence of services and potential impacts. The individual service providers contacted, and their response are shown in Table 3-2.

Table 3-2 Summary of utilities companies' infrastructure

| Service Provider | Response Received | Services Present |
| :--- | :---: | :---: |
| Aurora Telecom | Yes | No |
| BT | Yes | Yes |
| Eir | Yes | No |
| Enet | Yes | No |
| ESB Networks | Yes | Yes |
| Bord Gais | Yes | Yes |
| Virgin Media | Yes | Yes |
| Irish Water | Yes | Yes |
| FCC surface \& foul water | Yes | Yes |

### 3.7. Initial Transport Assessment

Traffic count surveys were carried out at six relevant junctions in May 2018. The surveys consist of pedestrian crossing counts, junction turning counts and queue counts at selected locations.

### 3.7.1. Link Analysis

A link analysis was carried out using the survey data. Two scenarios were assessed a Without Scheme scenario and a With Scheme scenario.

The assessment found that in the With Scheme scenario all of the road links would operate well within capacity, with Ratio of Flow to Capacity (RFC)s generally less than 0.6. A summary of these results is shown in Table 3-3 below.

Table 3-3 Link analysis results

| Link | Baseline Capacity (Veh/hr) | Capacity with Scheme (Veh/hr) | Peak | Flow (Veh/hr) | Baseline RFC | With Scheme RFC |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Harry Reynold's Rd. (Site 1 to Site 2) | 1300 | 900 | Am | 235 | 18\% | 26\% |
|  |  |  | PM | 279 | 21\% | 31\% |
| Harry Reynold's Rd. (Site 2 to Site 3) | 1300 | 900 | AM | 482 | 37\% | 54\% |
|  |  |  | PM | 324 | 25\% | 36\% |
| Harry Reynold's Rd. (Site 3 to Site 2) | 1300 | 900 | AM | 600 | 46\% | 67\% |
|  |  |  | PM | 374 | 29\% | 42\% |
| Harry Reynold's Rd. (Site 3 to Site 4) | 1300 | 900 | AM | 424 | 33\% | 47\% |
|  |  |  | PM | 363 | 28\% | 40\% |
| R132 <br> (Site 4 to Site 5) | 1530 | 900 | AM | 494 | 32\% | 55\% |
|  |  |  | PM | 513 | 38\% | 53\% |
| R132 <br> (Site 5 to Site 4) | 1300 | 900 | AM | 562 | 43\% | 62\% |
|  |  |  | PM | 358 | 28\% | 40\% |

### 3.7.2. Junctions Review

A review of the existing traffic conditions (site visit, traffic counts and online data) indicates that the existing roundabout at Harry Reynolds Road/ Dublin Street/ Hamilton Road is operating close to capacity during the peak hour periods. Two possible options were considered for this junction to cater for cyclists and other road users. These are discussed in further chapters.

The existing roundabout at Harry Reynolds Road/ Moylaragh Road has single lane entries on each arm. A review of the existing traffic conditions (site visit, traffic counts and online data) indicates that the existing roundabout is operating well within capacity during the peak hour periods. Two possible options were considered for this junction to cater for cyclists. These are discussed in further chapters

There are two existing signalised junctions (located at Chapel Street / Harry Reynolds Road and the Harry Reynolds / Drogheda Street) that are situated along the proposed route. The proposed scheme will have minor impacts on these junctions and it is not proposed to alter their operation in any major way.

## 4. Constraints

### 4.1. Engineering Constraints

A wide variety of data and information sources were used in identifying the engineering constraints including:

- Data and information obtained through consultations with Fingal County Council.
- Information obtained from public utility companies.
- Mapping data provided by Ordnance Survey Ireland under licence agreement.
- Topographical survey data.
- Road Safety Authority collision data.
- Route character information and road user behaviour collected as part observations recorded during site inspections.

The main constraints are discussed in the following sections.

### 4.2. Cross Section Options

Following initial site observations, three possible cross-section options were identified. The cross section options identified are shown in Figure 4-1, Figure 4-2 and Figure 4-3 below.

### 4.2.1. Link Type 1 - Two way cycle track and segregated footpath



Figure 4-1 Two way cycle track with segregated footpath

### 4.2.2. Link Type 2 - One way cycle lanes



Figure 4-2 One way cycle lanes
4.2.3. Link Type 3 - Two way cycle track and one way cycle track with segregated footpaths


Figure 4-3 Two way cycle track and one way cycle track with segregated footpaths
Figure 4-4 below shows the locations along the scheme where each link type is achievable. Cross-Section Option 2 is the narrowest with Option 3 being the widest. Cross-Section Option 2 is achievable along the full length of the route while Option 1 is achievable for all but a very short section. Option 3 is only achievable along Hamilton Road and a short section of Harry Reynolds Road.


Figure 4-4 Possible locations of link types along the proposed route

### 4.3. Land Ownership

The proposed facility will be constructed adjacent to the existing carriageway. The majority of these lands are under the control of Fingal County Council.

There are some isolated locations where the ownership of the lands must be further investigated. In particular, there are three areas of interest located to the south of Harry Reynolds Road at Stephenstown Industrial Estate; the first being the grassed area to the south of Casey Doors car park, the second being the grassed area to the north of Aravato Digital Services car park and finally the paved area adjacent to the footpath along the northern side of Harry Reynolds Road. Figure 4-5 below shows the location of these lands.

A second location where the ownership of lands requires further investigation is at the junction of Harry Reynolds Road and Drogheda Street. In order to tie into the existing cycle track the ownership of this land and masonry wall must be established. The location of this wall is shown in Figure 4-6.

The ownership of these lands will be examined in further detail within the next design stage of the project.


Figure 4-5 Land outside FCC ownership along Harry Reynolds Road


Figure 4-6 Landownership requiring further investigation at Drogheda Street junction

### 4.4. Environmental Constraints

### 4.4.1. Methodology

The environmental constraints assessment comprised a desktop study which focussed on the following key environmental topics; ecology / biodiversity, current / historic land-use, geology, hydrology, hydrogeology and flood risk.

The desk-based review was supplemented by a site walkover survey, which was carried out on $18^{\text {th }}$ May 2018 by an Atkins Environmental Consultant, along all accessible portions of the selected study area. The findings of the walkover survey were used to inform the environmental constraints assessment. This assessment represents a preliminary environmental review of the study area to inform the design process and supplementary environmental surveys may be required along the preferred route during the detailed design stage.

### 4.4.2. Ecological Constraints

The Bracken River, which flows in a northerly direction through the eastern portion of the study area, and the Bremore River, which flows in a general easterly direction through the northern portion of the study area are considered possible environmental constraints within the study area; however from an ecological perspective, neither of these rivers are hydrologically linked to Sites of International or National Importance.

The closest national site, Knock Lake proposed Natural Heritage Area (pNHA: Site Code: 001023), is located ca. 2km south west of the study area and the closest international site, River Nanny Estuary and Shore Special Protection Area (SPA: Site Code: 004158 ) is located ca. 4.25 km north of the study area, as presented in Figure 4-7.


Figure 4-7 Sites of National and International Importance (Source; NPWS 2018)

According to NBDC 2018, there has been a recent record of Japanese Knotweed (Fallopia japonica) within the town of Balbriggan, at No.45, Dublin Street, which is not in the vicinity of the proposed scheme.

During the site walkover there were no sightings of invasive species within the vicinity of the study area. There were a number of areas adjacent to the study area which had restricted access, and therefore could not be visually inspected for the presence of invasive species. However, as these areas are not within the boundary of the study area, it is anticipated that they will not be affected by the proposed scheme.

### 4.4.3. Current and Historic Land-use

During the site walkover, the dominant land use observed within the study area is residential and commercial, with some amenity grassland. According to EPA 2018 there are no EPA licenced facilities, waste facilities or Waste Water Treatment Plants (WWTP) within the study area.

Based on a review of OSI mapping including historic mapping, a Mill is identified to the east of the study area, with a Mill Pond historically located adjacent to the Mill. However the pond appears to have been filled in, and the area is now covered with Made Ground and consists of amenity grassland, with a river to west of the study area. The entire study area was historically dominated by agricultural land. The general vicinity of the study area was generally developed between 1913 and 1995. The Stephenstown Industrial Park and Balbriggan Business Park were also developed during this time and were developed further between 1995 and 2000.

### 4.4.4. Geology and Soils

According to the GSI (2018) there are a variety of soil types beneath the vicinity of the study area. The eastern portion of the study area is dominated by Made Ground and deep well drained mineral soils. The central and north-western portions of the study area comprise Made Ground with minor portions of alluvial soils. The southwestern portion of the study area is dominated by soils which are poorly drained (mainly acidic).

Bedrock beneath the general vicinity of the study area is dominated by Andesite, pillow breccia, mudstone and tuff of the Belcamp Formation with a portion to the south-east comprising laminated blue-grey siltstone, and sandstone of the Skerries Formation (GSI, 2018).

### 4.4.5. Hydrology

The Bracken River flows through the eastern portion of the study area. The river flows in a northerly direction before discharging to Balbriggan Harbour ca. 0.7 km upstream of the study area. The Bremore River flows in an easterly direction through the northern portion of the study area, before discharging to coastal waters ca. 0.45 km upstream (east) of the study area. Refer to Figure $4-8$. Neither of the Rivers have been assigned a surface water ecological status by the EPA (2010-2015).


Figure 4-8 Key surface water features (source; EPA 2018)

### 4.4.6. Hydrogeology

The GSI provides a methodology for aquifer classification based on resource value (regionally important, locally important and poor) and vulnerability (extreme, high, moderate or low).

The bedrock aquifer within the vicinity of the site is classified as ' $L m^{\prime}$ ', a locally important aquifer which is generally moderately productive (GSI, 2018). There are no gravel aquifers beneath the study area or within 2km (GSI, 2018).

According to GSI, 2018, the groundwater vulnerability beneath the central and northern portions of the site are classified as 'low' with the northern and small a portion of the south eastern areas being classified as 'moderate'. The south eastern corner of the study area has been classified as 'high' vulnerability, indicating shallow bedrock in this portion of the study area; therefore, groundwater is vulnerable to potential contamination in this area.

### 4.4.7. Flood Risk Screening

Relevant best practice guidance "The Planning System and Flood Risk Management - Guidelines for Planning Authorities" (DEHLG, 2009) sets out a risk-based sequential approach to flood risk assessment. Three key stages are identified as follows;

Stage 1 - Flood Risk Identification - To identify whether there may be any flooding or surface water management issues related to a plan area or proposed development site that may warrant further investigation.

## Stage 2 - Initial Flood Risk Assessment

## Stage 3 - Detailed Flood Risk Assessment

This PFRA has been carried out in accordance with relevant best practice guidance (DEHLG, 2009) and comprised the completion of a 'Stage 1 - Flood Risk Identification' screening assessment.

The following key sources of potential flooding associated with the proposed scheme have been identified;

- Rivers / streams / surface water courses;
- Heavy rainfall and associated surface water ponding; and,
- Coastal /tidal flood waters;

There has been no historic flooding within the study area. The Fingal East Meath (FEM) Flood Risk Assessment and Management Study (FRAMS), predictive flood risk maps suggest that flooding in the vicinity of the Bremore River has a 1 in 10 probability in any given year, while flooding in the vicinity of the Bracken River has a 1 in 100 probability in any given year (OPW, 2018). Refer to Figure 4-9.


Figure 4-9 Flood Extents Map (Source; OPW 2018)
It is not envisaged that the proposed pedestrian and cyclist scheme would have any adverse impact on flooding along the path, based on the following considerations: -

- The proposed pedestrian and cyclist scheme shall be designed to avoid key low-lying areas identified during the detailed design stage which may be at potential flood risk;
- The preliminary drainage design comprises the following key elements;
- Adequate drainage has an enormous impact on the quality and safety of cycling facilities. Drainage will be installed to the desired standard and will be compliant with Sustainable Urban Drainage Systems (SuDs) as set out in the Greater Dublin Strategic Drainage Study (2005). Where possible the existing drainage infrastructure is to be maintained.
- Cycle friendly drainage, such as side entry gullies, will be incorporated where appropriate.

The localised change in land surface and the improved drainage systems may result in a minor increase in rainfall run-off rates. However, such increases will be minor relative to existing conditions, and are unlikely to
have a significant impact on the existing hydrological regime along the route of the proposed scheme. Therefore, it is not envisaged that the proposed development will pose any significant potential flood risk to the surrounding lands, properties or the surrounding road network.

### 4.5. Constraints Due to Human Beings

The proposed route will not add to operational sources of noise or air pollution from e.g. vehicular traffic but will assist in promoting more sustainable transport with associated reductions in such emissions.

If necessary, environmental disciplines such as noise, air, etc. will be assessed further as the project proceeds as these do not significantly inform the design above and beyond constraints informed by proximity of the route to residential properties etc.

### 4.6. Tree and Hedgerow (Stage 1)

There will be no impact on any trees with a protection order by the proposed scheme.
The removal of some immature tress along the route is foreseen particularly along Harry Reynolds Road (sections 4, 5 and 6). Efforts will be made mitigate against the impact of the removal of these trees and new trees will be provided in alternative locations along the route. The additional cycle provision through the park lands to the south of the scheme may also necessitate removal of some trees. Should this occur all efforts will be made to minimise the extents of the tree removal.

A Tree and Hedgerow survey will be undertaken as part of the preliminary design.

### 4.7. Architectural and Built Heritage

There are three protected structures within 500 metres of the proposed route.

- St Peter's \& Paul's Church. The structure is a mid $19^{\text {th }}$ century Roman Catholic Church
- Parochial House.

The structure is a turn of the $20^{\text {th }}$ century parochial house serving St Peter's \& Pauls Church

- Former Corn Mill.

The structure is a mid $19^{\text {th }}$ century four storey mill, converted to an apartment block.
The locations of these structures is shown in Figure 4-10 below.
The other notable elements of the Built Heritage are the cemetery adjacent to the Harry Reynolds Road/ Chapel Street/ Nual Road junction and the town monument in the centre of the roundabout on Dublin Street.


Figure 4-10 Locations of protected structures

## 5. Stakeholder Consultation

### 5.1. Fingal County Council

FCC Transportation department have consulted with the various internal departments within FCC. The comments recorded from each of the departments are detailed below.

### 5.1.1. Parks

An on-site meeting was undertaken with undertaken on the $4^{\text {th }}$ September 2018. The key issues arising from this meeting are summarised below:

Planting at roundabout at Dublin st church car park. In general, no overall objection but want to know soon so that maintenance work in the area can be planned for or left out.

Open space area near church car park. There are a lot of plans for development of the park - including development of a possible a running track and a skate park. The opportunity of linking the proposed cycleway/footpath into the running track should be considered.

Removal Trees along harry Reynolds road. As many compensatory trees as possible should be planted, although this may be only half the number removed. The current trees provide an avenue of trees and are soft landscaping to the cold environment of concrete, blacktop and houses. FCC does not maintain any grass along Harry Reynolds road, only the grass in front of FCC open spaces.

North of the Garda station, the preference would be to maintain as many trees as possible.
Trees in Moylaragh Open Space.It is possible to put cycle-lane on southern side of open space away from current footpath. At least 3 m width buffer needed from kerb and cycleway for maintenance. Existing open space was not well developed and needs further development. Some trees planted are not long life.

### 5.1.2. Planning

Planning provided comments on the emerging concept options, these comments are summarised below and consideration of these comments has been taken into the design process and will be given further consideration during the next stage. The planning divided the scheme into nine sections, which are similar to the sections detailed in the road infrastructure review; the difference between the sections is that planning divided Section 1 into two section, with section 1 being just from Drogheda Street to the junction with Ashfield Rise

Table 5-1 FCC Planning Section Comments

| Planning <br> Section | Planning Comment | Response |
| :--- | :--- | :--- |
| 1 | Provide 2-way cycleway within grass verge <br> on the northern side. | This option was considered as part of the Multi- <br> Criteria Analysis that follows. |
| 2 | Provide a 2-way cycleway in the open <br> space between the railing and the trees. | Consideration of this option was given, to provide <br> this option a number of gaps within the wall would <br> need to be provided to allow access to/from the <br> cycleway. This would require cyclists to cross the <br> footpath at numerous locations, increasing <br> conflicts for pedestrians and cyclists. |
| 3. | Provide a 2-way cycleway in the open <br> space between the railing and the trees. | Consideration of this option was given, however <br> there is limited space between the trees and the <br> railing at this location and this would place the <br> cycleway too close to existing residential <br> properties. |
| 4. | Align a 2-way cycleway inside the southern <br> edge of the public open space | Consideration of this option was given. There are <br> a number of options for this section and a |


| Planning <br> Section | Planning Comment | Response |
| :--- | :--- | :--- |
| 6 | Cycleways should be located within the <br> existing wide grass verge on the west side | Consideration was given to this option, however <br> determine the preferred option. <br> as this side is not as accessible to the local <br> residential population is was not the preferred side <br> for a 2-way facility. |
| 7 | A separate track may be needed on both <br> sides of the road. | This option was included as part of the Multi- <br> Criteria Analysis. |
| 8 | Align track along narrow road serving the <br> church car park and off-road up to the <br> R132 | This option was included as part of the Multi- <br> Criteria Analysis. |
| 9 | The existing segregated cycleways along <br> both sides of the road should be <br> maintained | This option was included as part of the Multi- <br> Criteria Analysis. |

### 5.2. National Transport Authority

FCC have consulted with NTA as part of this stage of the project and a summary of their comments is included in the table below.

Table 5-2 NTA Comments

| NTA Comments | Response |
| :--- | :--- |
| NTA stated that the existing cycle tracks on <br> Hamilton Road are generally adequate but the <br> entrance to the school could be upgraded. | This option was included as part of the Multi-Criteria <br> Analysis. |
| NTA stated a preference for not providing a new <br> signal controlled junction to replace the existing <br> roundabout junction at Dublin Road/Hamilton Road <br> but the junction could be configured to a cycle <br> friendly roundabout as per National Cycle Manual. | Both signalised junction and fully cycle friendly <br> roundabout options were included as part of the <br> MCA. |
| NTA stated a preference for 1 way cycle tracks on <br> both sides of the road whenever possible. | This option was included as part of the Multi-Criteria <br> Analysis for all sections. |
| NTA suggested that it may be better to widen the <br> existing footpath in the park at Moylaragh or to <br> increase traffic calming on Chieftain's Drive rather <br> than constructing new cycle tracks. | These options were included as part of the MCA. <br> The exact route of new cycle tracks through the park <br> will be determined through consultation with FCC <br> Parks Department as part of the preliminary design. |

A further on-site meeting was held with the NTA, Atkins and FCC on the $12^{\text {th }}$ September 2018. At this meeting the scheme was reviewed in detailed and the following points were agreed:

- A new toucan crossing should be provided on Hamilton Road, adjacent the entrance to the Town Park.
- That environmental improvements would be considered for the access lane to the schools on Hamilton Road.
- That environmental improvements would be considered for the laneway from Curran Park to Hamilton Road
- The preferred option for the section of Harry Reynolds Road to the north of the Garda Station would be the provision of a two-way cycleway on the northern side of the carriageway.
- The preferred solution at Moylaragh would be the provision of permeability links within Moylaragh. Cyclists along the Harry Reynolds Road would be signed to use Chieftains Drive. Details of these permeability links are provided within Appendix C.


## 6. Options Assessment

### 6.1. Methodology

In order to assess and compare the various options available along each part of the route a Multi Criteria Analysis process was implemented. The scheme was divided into 5 distinct sections for links and 2 sections for junctions and analysed on that basis. Each section was assessed individually but with cognisance of the adjoining sections and a preferred option established for each. The combination of these preferred options is the preferred overall option for the scheme.

### 6.2. Assessment Criteria

A number of criteria were established with reference to the National Cycle Manual and Common Appraisal Framework accounting for the benefits and impacts on cyclists, pedestrians and other road users as well as on the wider community. The main criteria headings are included below:

### 6.2.1. Design Context

These assessment criteria primarily relate to the five needs of the cyclists as set out in the National Cycle Manual but also take account of other vulnerable road users. The criteria in this category are:

- Safety
- Directness
- Coherence
- Attractiveness
- Comfort


### 6.2.2. Community Context

The interests of the local community are also considered within the assessment criteria. These are as follows:

- Impact on business.
- Impact on residents.
- Operational impacts.


### 6.2.3. Delivery Context

The consideration of risk in terms of construction costs and programme are also considered within the assessment criteria. These are as follows:

- Budget risk.
- Programme risk.


### 6.2.4. Sub-Criteria

The full definition of all items considered under each criterion is shown in the table below. Each of the subcriteria was accounted for when comparing each option.

As safety is at the core of all good designs it is considered to be one of the most critical criteria and has been weighted higher than other criteria.

Level of service has been weighted the highest as the purpose of the scheme is to provide the highest possible level of service.

Table 6-1 MCA Assessment Criteria

| Context | Main Criteria | Sub Categories | Weighting |
| :---: | :---: | :---: | :---: |
| Design | Safety | - Traffic volume and speed <br> - Vehicle conflicts - links <br> - Vehicle conflicts - junctions <br> - Pedestrian conflicts <br> - Pedestrian safety | 15 |
|  | Directness | - Transitions between links <br> - Treatment of side roads and junctions <br> - Ability to overtake | 10 |
|  | Coherence | - Route continuity and consistency <br> - Route legibility <br> - Obstructions (illegal parking) | 10 |
|  | Attractiveness | - Integration <br> - Cycling experience <br> - Contribution to urban design <br> - Impact on heritage and landscape | 10 |
|  | Comfort | - Provision of adequate width <br> - Maintain cyclist progress <br> - Suitability for all users | 10 |
|  | Level of service | - Progression of cyclists <br> - Quality of facility | 20 |
| Community | Business impact | - Property access <br> - Loading <br> - Parking | 10 |
|  | Residential impact | - Property access <br> - Impact on land / land acquisition <br> - Traffic management impacts on journey times | 10 |
|  | Operational impact | - Impact on junctions <br> - Impact on maintenance cost | 10 |
|  | Environmental | - Impact on the surrounding environment | 10 |
| Delivery | Capital cost | - Construction cost <br> - Land / property acquisition cost <br> - Overall scheme cost | 10 |
|  | Programme risks | - Land / property acquisition legal process <br> - Construction risks including utilities | 10 |

### 6.2.5. Scoring Procedure

Each of the design options are assessed against the criteria specified above in a performance matrix which indicates how each option performs against the criteria and in comparison to the other design option. The assessment is, therefore, comparative and the scoring reflects the performance of each option against other options.

Each criterion is assessed on a five-point colour coded scale as presented in the table below. This scale rates how each option satisfies a particular criterion.

Table 6-2 Scoring Scale

| Colour Coding | Rank Description |
| :---: | :---: |
|  | Very Positive |
|  | Slightly Positive |
|  | Neutral |
|  | Slightly Negative |
|  | Very Negative |

### 6.3. Link Assessments

As discussed, for the purposes of the assessment, the proposed scheme has been divided into various sections, with the sections for the link assessment shown in Figure 6-1 below. Varying options were developed for each link section and each option assessed under the criteria set out above.


Figure 6-1 Sections for Link Assessment

### 6.3.1. Link Section 1

Link Section 1 covers Hamilton Road, The road links the four arm roundabout on Dublin Street and the three arm roundabout on Castle Park Avenue. Three options were identified for this section and are set out below

### 6.3.1.1. Option 1 - Do Minimum

This option would retain the existing cycle and pedestrian facilities along this section of road. The existing toucan crossing may be relocated to better serve the existing schools to the southern side of the road. This option would require minimal works to implement.

Following on-site meetings, it was agreed that this option would include the provision of the toucan crossing adjacent to the entrance to Town Parks and that a two way cycle facility would be provided to link the new crossing to the entrance to the school.


Figure 6-2 Option 1 - Do Minimum

### 6.3.1.2. Option 2 - Two Way Segregated Cycle Track

For this option, a two way segregated cycle track would be provided along the northern side of Hamilton Road while the existing facilities to the southern side of the road would remain in place. The cycle track would be 3.0 m in width. Pedestrians will be catered for by a 2.0 m wide footpath to the rear of the cycle track. The northern location of these facilities would allow greater access to the large residential areas to the north of the road. A toucan crossing will be provided in advance of the education centres, Ardgillan Community College and Braken Educate Together. The figure below shows the location of the potential two way cycle facility and relocated toucan crossing. This option would require construction of the new cycle track and footpath within the existing grass verges, cycle track and footpaths but would not require alterations to the kerblines.


Figure 6-3 Option 2 - Two Way Segregated Cycle Track

### 6.3.1.3. Upgraded One Way Cycle Tracks Both Sides

This option would upgrade the existing cycle tracks on both sides to provide 2.0 m wide cycle tracks. The existing footpaths would also be upgraded to 2.0 m in width. Upgrading of these cycle tracks would be carried out by removing the existing grass verges and widening into the grassed areas to the rear of the existing footpath but would not require relocation of kerblines. The existing crossing location in advance of the Castle Park Avenue roundabout could be relocated as in Option 1. The figure below shows the potential upgraded cycle tracks and toucan crossing


Figure 6-4 Option 3 - Upgraded One Way Cycle Tracks Both Sides

### 6.3.1.4. Link Section 1 - MCA

The Multi Criteria Analysis and comparison between each option for Link Section 1 is summarised in the table below.

Table 6-3 Link Section 1 - MCA Comparison Matrix

| Context | Criteria | Option 1 | Option 2 | Option 3 |
| :---: | :---: | :---: | :---: | :---: |
| Design | Safety* | Provides a safe environment for pedestrians and cyclists. <br> The channels of travel are well defined and a buffer is provided between the facility and the carriageway. | Provides a safe environment for pedestrians and cyclists. <br> Segregated cycle track and footpath minimises conflicts between cyclists and pedestrians. | Provides a safe environment for pedestrians and cyclists. <br> The channels of travel are well defined and a buffer is provided between the facility and the carriageway. |
|  | Directness | Provides as direct a route as possible along the section. | Provides as direct a route as possible along the section. | Provides as direct a route as possible along the section. |
|  | Coherence | A highly legible route which is well defined and free of obstructions. | A highly legible route which is well defined and free of obstructions. | A highly legible route which is well defined and free of obstructions. |
|  | Attractiveness | Provides a wellintegrated cycle and pedestrian route. | Provision is somewhat isolated from southern area as it is on northern side of carriageway. | Provides a wellintegrated cycle and pedestrian route. |
|  | Comfort | Provides a comfortable facility, suitable for all users. | Provides comfortable facility, suitable for all users. | Provides comfortable facility, suitable for all users. |
|  | Level of Service* | Separated cyclist and pedestrian channels of travel not segregated but providing adequate level of service. <br> Conflicts between modes is minimised allowing greater service level. | Segregated cycle and pedestrian facility allows for level of service $A$. <br> Conflicts between modes is minimised allowing greater service level. | Separated cyclist and pedestrian channels of travel allows for level of service A. <br> Conflicts between modes is minimised allowing greater service level. |
| Community | Business impact | No impact on businesses. | No impact on businesses. | No impact on businesses. |
|  | Residential impact | No impact on residents. | Minimal impact on residents during construction. | Minimal impact on residents during construction. |
|  | Operational impact | Facility will require routine maintenance. | Facility will require routine maintenance. | Facility will require routine maintenance. |
|  | Environmental | Facility has no impact on environment as is currently in place. | Facility will have some impact on surrounding environment during construction. | Facility will have some impact on surrounding environment during construction. |
|  |  |  | Proposed facility requires widening into grassed verges. | Proposed facility requires widening into grassed verges |


| Context | Criteria | Option 1 | Option 2 | Option 3 |
| :---: | :---: | :---: | :---: | :---: |
| Delivery | Capital cost | Minimal additional costs to scheme. | 3 m wide cycle track and 2 m wide footpath requires widening into grassed verge. <br> Additional capital costs may not be justified given existing facilities. | 3 m wide cycle track and 2 m wide footpath requires widening into grassed verge. <br> Additional capital costs may not be justified given existing facilities. |
|  | Programme risks | Minimal risks to programme delivery. | Programme could suffer due to works requirements. <br> Potential conflicts with utilities due to widening. | Programme could suffer due to works requirements. <br> Potential conflicts with utilities due to widening. |
| Ranking |  | 1st | 3rd | 2nd |

### 6.3.1.5. Link Section 1 - Preferred Option

Option 1 is the preferred option in this location. The existing facility delivers a well-designed provision which performs well against the design criteria while requiring little capital cost. The facility also has a marginal impact on the community and does not raise any concerns towards budgets or programme delivery. A new toucan crossing will be provided and a two-way cycleway will be provided between the toucan crossing and the access to the school. The figure below shows the preferred route for Link Section 1.


Figure 6-5 Link Section1 - Preferred Option

### 6.3.2. Link Section 2

Section 2 covers the one-way exit road from the public car park adjacent to St Peter and Pauls Church. Three possible options were considered for this section and are described below.

### 6.3.2.1. Option 1 - Do Nothing

This option would retain the existing footpath link that runs along the perimeter of the car park between the Dublin Street roundabout and the car park exit road. No cycle facilities would be provided along this section but the existing narrow, unsegregated cycle tracks along the L1360 would be maintained on both sides along with the section of cycle track on one side of the Harry Reynolds Road. Two new toucan crossings would also be required. The proposed option is shown in the figure below.


Figure 6-6 Option 1 - Do Nothing

### 6.3.2.2. Option 2 - Two Way Segregated Cycle Track.

A two-way segregated cycle track will be provided along the northern side of the car park exit road. Cycle provision will be by a 3.0 m wide track with a 2.0 m wide footpath also provided to the rear of the cycle track. A 2.5 m footpath will provide a link from this facility to the R 132 roundabout enabling progression. A raised zebra crossing would be provided to link the new facility to the existing footpath that runs along the perimeter wall of the existing car park. This footpath would be upgraded to the same standard including a 3 m wide two-way cycle track $2 m$ wide footpath. A new toucan crossing facility would be provided along the Harry Reynolds Road to the west of the car park exit road. The proposed facilities are shown in the figure below.


Figure 6-7 Option 2 - Two-Way Segregated Cycle Track

### 6.3.2.3. Option 3 - One Way Cycle Tracks Both Sides

One way cycle tracks will be provided along both sides of the car park exit road. Cycle provision will be gained by 2 m wide tracks while 2 m wide footpaths will also be provided to the rear of the cycle tracks. A raised zebra crossing would be provided to link the new facility to the existing footpath that runs along the perimeter wall of the existing car park. This footpath would be upgraded to include a 3 m wide two-way segregated cycle track and 2 m wide footpath. The existing priority junction between the car park exit road and Harry Reynolds Road would be upgraded to a new signalised junction with toucan crossings. The proposed layout is shown in the figure below.


Figure 6-8 Option 3 - One Way Cycle Tracks Both Sides

### 6.3.2.4. Link Section 2 - MCA

The Multi Criteria Analysis and comparison between each option for Link Section 2 is summarised in the table below.

Table 6-4 Link Section 2 - MCA Comparison Matrix

| Context | Criteria | Option 1 | Option 2 | Option 3 |
| :---: | :---: | :---: | :---: | :---: |
| Design | Safety* | Provides a relatively safe environment for cyclists travelling westwards but not for cyclists travelling eastwards who have intermittent facilities and need to cross at two roundabouts. <br> Cyclists may use existing footpath link increasing conflicts with pedestrians. | Provides a safe environment for pedestrians and cyclists with appropriate crossings. <br> Segregated cycle way and footpath minimises conflicts between cyclists and pedestrians. | Provides a safe environment for pedestrians and cyclists with appropriate crossings. <br> Segregated cycle way and footpath minimises conflicts between cyclists and pedestrians. |
|  | Directness | Not a desirable route as it adds approximately 300 m to the overall length of the scheme. | Provides as direct a route as possible along the section. | Provides as direct a route as possible along the section. |
|  | Coherence | Legible along much of the route but no cycle facilities present on western side of Harry Reynolds Road. | A highly legible route which is well defined and free of obstructions. | A highly legible route which is well defined and free of obstructions. |
|  | Attractiveness | Does not provide an enjoyable cycling experience due to additionl length and variations in level. | Provides a wellintegrated cycle and pedestrian route on the obvious desire line. | Provides a wellintegrated cycle and pedestrian route on the obvious desire line. |
|  | Comfort | Slows cyclist progress due to length. <br> Requires additional effort due to level variations. | Provides a comfortable facility, suitable for all users. <br> Lessens overall route length. | Provides a comfortable facility, suitable for all users. <br> Lessens overall route length. |
|  | Level of service | Narrow cycle tracks with no segregation to footpath and lack of crossings at roundabouts means that level of service can't be achieved. | Separated cyclist and pedestrian channels of travel allows for level of service $A$. <br> Conflicts between modes is minimised allowing greater service level. | One way cycle tracks and pedestrian footpaths minimise conflicts between modes allowing greater service level. |
| Community | Business impact | No impact to businesses. | Potential impact on Casey Doors lands under long term lease from FCC. | Potential impact on lands under ownership of Casey Doors and Jack |



### 6.3.2.5. Link Section 2 - Preferred Option

Option 2 is the preferred option in this location. It will provide a safe, attractive route which with a very high level of quality of service through this section. While Options 2 and 3 score similarly in many categories, Option 2 does not require a new signalised junction at Harry Reynolds Road and the car park exit road, thus reducing impacts to traffic in the area and reducing the overall cost. The figure below shows the preferred route for Link Section 2.


Figure 6-9 Link Section 2 - Preferred Option

### 6.3.3. Link Section 3

Section 3 extends along Harry Reynolds Road in a northward direction. In the southern section the route passes through Balbriggan Business Park. The single carriageway has an overall width of approximately 7.0m with footpaths running adjacent to the carriageway on either side. There is an existing cycle track along the western side of the road to the rear of on street parking. A speed limit of $60 \mathrm{~km} / \mathrm{h}$ applies through this part of the section.

The central section extends from the Balbriggan Business Park to the signal controlled junction at Chapel Street. The single carriageway has an overall width of approximately 9.0 m . A footpath runs adjacent to the carriageway on the eastern side only. A number of access junctions to industrial and residential developments are present along the carriageway. A $60 \mathrm{~km} / \mathrm{h}$ speed limit also applies to this part of the section.

The northern section extends between the roundabout junction at the northern end of Harry Reynolds Road and the Chapel Street junction. There are existing one way cycle tracks on the eastern side of the carriageway extending approximately 90 m from the Chapel Street junction. The single carriageway has an overall width of approximately $9.0-10.0 \mathrm{~m}$. Footpaths run adjacent to the carriageway on either side with a buffer provided by means of a grass verge also on both sides. The eastern verge is lined with immature trees. A number of access junctions to residential streets are present along the carriageway. This section of the scheme has a 50 kph speed limit in place. Four possible options were considered for this section and are discussed below.

### 6.3.3.1. Option 1 - Do Nothing

No provision would be made for cyclists aside from the existing provision at the junction of Harry Reynolds Road and Chapel Street, as highlighted in the figure below. Outside of this provision, cyclists must share the carriageway with vehicular traffic while pedestrian facilities will remain as is.


Figure 6-10 Option1 - Do Nothing

### 6.3.3.2. Option 2 - Two Way Segregated Cycle Track.

A two way segregated cycle track would be provided along the eastern side of Harry Reynolds Road. The eastern side of the carriageway has been selected as the bulk of the population in the area lie to the east. Cycle provision will be gained by a 3 m wide track with a 2 m footpath also provided to the rear of the cycle track. The existing kerbs would be relocated with new carriageway widening required on the western side of the road. Grass verges would be removed to accommodate the new facilities. Some alterations to the existing signalised junction at Chapel Street will be required to facilitate the new layout. The proposed option is shown in the figure below.


Figure 6-11 Option 2 - Two Way Segregated Cycle Track

### 6.3.3.3. Option 3 - One Way Cycle Tracks Both Side

One way cycle tracks would be provided along both sides of Harry Reynolds Road. Cycle provision will be gained by 2 m wide raised adjacent cycle tracks with 2 m wide footpaths also provided to the rear of the cycle tracks. The existing carriageway would be narrowed and kerblines relocated to accommodate the proposed cycle tracks and footpaths. Grass verges would be removed along the length of the road. The proposed cycle tracks would tie into the existing cycle facilities at the Chapel Road junction with no alterations proposed. The proposed option is shown in the figure below.


Figure 6-12 Option 3-One Way Cycle Tracks Both Sides

### 6.3.3.4. Option 4 - On-Road Cycle Lanes.

On road cycle tracks would be provided along both sides of Harry Reynolds Road. Cycle provision will be gained by 2 m wide lanes on carriageway. The existing carriageway will be widened to allow for the inclusion of the cycle lanes with existing kerbs relocated. Pedestrians will be catered for utilising the existing footpaths. The proposed cycle lanes and footpaths would be segregated by means of a 0.5 m wide buffer. Grass verges would be removed along the length of the road. The proposed on-road cycle lanes would tie into the existing layout at the Chapel Street junctions with no alterations proposed. The proposed option is shown in the figure below.


Figure 6-13 Option 3 - On Road Cycle Tracks

### 6.3.3.5. Link Section 3 - MCA

The Multi Criteria Analysis and comparison between each option for Link Section 3 is summarised in the table below.

Table 6-5 Link Section 3 - MCA Comparison Matrix

| Context | Criteria | Option 1 | Option 2 | Option 3 | Option 4 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Design | Safety* | Does not provide a safe environment for cyclists as they must share the carriageway with vehicular traffic. | Provides a safe environment for pedestrians and cyclists. <br> Segregated cycle way and footpath minimises conflicts between cyclists and pedestrians. | Provides a safe environment for pedestrians and cyclists. <br> The channels of travel are well defined. | Provides a designated area for cyclists delivering improved safety but with no physical segregation. |
|  | Directness | Does not provide opportunity to overtake as environment is shared with vehicular traffic. | Provides as direct a route as possible along the section. | Provides as direct a route as possible along the section. | Provides as direct a route as possible along the section. |
|  | Coherence | Route does not provide continuity with the rest of the scheme. | A highly legible route which is well defined and free of obstructions. | A highly legible route which is well defined and free of obstructions. | A highly legible route but may be subject to encroachment and illegal |


| Context | Criteria | Option 1 | Option 2 | Option 3 | Option 4 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Community |  | There is potential for obstructions throughout. |  |  | parking by vehicles. |
|  | Attractiveness | Option is not attractive to cyclists. <br> Design does not contribute to the urban design. | Provision is somewhat isolated on eastern side of carriageway. | Provides a wellintegrated cycle and pedestrian route serving all of the surrounding area. | Provision is not as attractive as segregated cycle tracks. |
|  | Comfort | No provision of cyclist own space cyclist may feel uncomfortable interacting with vehicles. | Provides a comfortable facility, suitable for all users. <br> Cyclist progression may be reduced due to necessity of crossing points. | Provides a comfortable facility, suitable for all users. <br> Cyclist progression is maintained throughout the section with minimal crossing points. | Sharing the carriageway with vehicles is less comfortable for many cyclists. <br> Surface is subject to greater levels of damage as it is shared with vehicles. |
|  | Level of service | No level of service provided. | Separated cyclist and pedestrian channels of travel allows for level of service A. <br> Conflicts between modes is minimised allowing greater service level. | One way cycle tracks and pedestrian footpaths allow conflicts between modes is minimised. <br> Raised adjacent tracks allowing greater service level as they provide segregation and avoid delays. | On road cycle tracks provide a good level of service but may be subject to delays etc due to vehicles in the cycle lane. |
|  | Business impact | No impact on businesses | Minor impact to businesses in general through the Business Park area only. | Minor impact to businesses in general through the Business Park area only | Minor impact to businesses in general through the Business Park area only |
|  | Residential impact | No residential impact. | No residential impacts. | No residential impacts. | No residential impacts. |
|  | Operational impact | Some very minor operational impacts possible due to presence of increased cyclists on road with from adjacent new facilities. | Some operational impacts due to need to alter existing junction at Chapel Street. | Minor <br> operational <br> impacts possible <br> due to narrowing <br> of carriageway <br> but no <br> alterations to <br> Chapel Street <br> junction <br> required. | Minor <br> operational impacts possible due to narrowing of carriageway but no alterations to Chapel Street junction required. |


| Context | Criteria | Option 1 | Option 2 | Option 3 | Option 4 |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Environmental | No impact to environment. | Upgrades to facility will have marginal impact on surrounding environment during construction and some existing trees will be removed. | Upgrades to facility will have marginal impact on surrounding environment during construction and some existing trees will be removed. | Upgrades to facility will have marginal impact on surrounding environment during construction and some existing trees will be removed.. |
| Delivery | Capital cost | No additional costs to scheme. | Significant cost implications to widening of carriageway on western side with possibility of utility diversions etc. | Costs associated with kerb relocation and construction of new cycle tracks/footpaths. <br> No carriageway widening required. | Significant cost implications to widening of carriageway on both sides with possibility of utility diversions etc. |
|  | Programme risks | No risks to programme delivery. | Programme could suffer due to additional difficulties because of widening. <br> Potential conflicts with utilities. | Minor risk to programme as a result of unknown services etc but minimised due to raised adjacent construction. | Programme could suffer due to additional difficulties because of widening. <br> Potential conflicts with utilities.. |
| Ranking |  | $4^{\text {th }}$ | $2^{\text {nd }}$ | 1 st | $3{ }^{\text {rd }}$ |

### 6.3.3.6. Link Section 3 - Preferred Option

Option 3 is the preferred option in this location. The provision of new raised adjacent cycle tracks on both sides along the entire section will provide a safe, attractive and accessible route for all road users. It will have minimal impact on the surrounding businesses and has no impact on any residences while providing a high quality of service. It is also advantageous from a delivery point of view as capital costs and risks are minimised in comparison to other options.


Figure 6-14 Link Section 3 - Preferred Option

### 6.3.4. Link Section 4

Section 4 extends from the junction between Harry Reynolds Road and Drogheda Street (R132) in a southwesterly direction to the roundabout junction where Harry Reynolds Road turns southward. The single carriageway has an overall width of approximately 6.5 m . Footpaths run adjacent to the carriageway on either side with a buffer provided by means of a grass verge. The southern verge is generally lined with immature trees. This section of the scheme has a posted speed limit of 50 kph . Four possible options were considered for this section and are discussed below.

### 6.3.4.1. Option 1 - Do Nothing

No provision would be made for cyclists aside from the existing provision at the junction of Harry Reynolds Road and Drogheda Street, as shown in the figure below. Outside of this provision cyclist must share the carriageway with vehicular traffic. Pedestrians provision would remain as is.


Figure 6-15 Option 1 - Do Nothing

### 6.3.4.2. Option 2 - Two Way Segregated Cycle Track

A two way segregated cycle track would be provided along the northern side of Harry Reynolds Road. The northern side of the carriageway has been selected to provide the facility as there are fewer access points resulting in a less interruptions for cyclists. Cycle provision would be a 3 m wide track with a 2 m footpath also be provided to the rear of the cycle track. The proposed facilities would be constructed without relocating any kerbs where possible resulting in the removal of the existing grass verge on the northern side of the road. The existing junction between Harry Reynolds Road and Dublin Road could be upgraded with the existing two-way cycle track tying into the proposed new one. The proposed option is shown in the figure below.


Figure 6-16 Option 2 - Two Way Segregated Cycle Track

### 6.3.4.3. Option 3 - One Way Cycle Tracks Both Sides

One way cycle tracks would be provided along both sides of Harry Reynolds Road. Cycle provision would be $2 m$ wide raised adjacent cycle tracks with $2 m$ wide footpaths also provided to the rear of the cycle tracks. The existing kerbs would be kept in place where possible with the facilities constructed over the existing grass verge and footpaths on both sides. However, some areas may require narrowing of the existing carriageway and associated kerb relocation. The existing junction between Harry Reynolds Road and Dublin Road could be upgraded with the existing two-way cycle track tying into the proposed new cycle tracks on both sides. The proposed option is shown in the figure below.


Figure 6-17 Option 3-One Way Cycle Tracks Both Sides

### 6.3.4.4. Option 4 - On Road Cycle Lanes.

On road cycle tracks would be provided along both sides of Harry Reynolds Road. The existing carriageway would be widened to allow for the inclusion of 2 m wide cycle lanes. This widening will be catered for by utilising the green area to the north of the carriageway Pedestrians will be catered for utilising the existing footpaths. The cycle lanes and footpaths will be segregated by means of a minimum 0.5 m buffer. The existing junction between Harry Reynolds Road and Dublin Road could be upgraded with the existing two-way cycle track tying into the proposed new cycle lanes which would be required to ramp up to the same level. The proposed option is shown in the figure below.


Figure 6-18 Option 4-On Road Cycle Lanes

### 6.3.4.5. Link Section 4 - MCA

The Multi Criteria Analysis and comparison between each option for Link Section 4 is summarised in the table below.

Table 6-6 Link Section 4 - MCA Comparison Matrix

| Context | Criteria | Option 1 | Option 2 | Option 3 | Option 4 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Design | Safety* | Does not provide a safe environment for cyclists as they must share the carriageway with vehicular traffic. | Provides a safe environment for pedestrians and cyclists. <br> Segregated cycle way and footpath minimises conflicts between cyclists and pedestrians. | Provides a safe environment for pedestrians and cyclists. <br> The channels of travel are well defined. | Provides a designated area for cyclists delivering improved safety but with no physical segregation. |
|  | Directness | Does not provide opportunity to overtake as environment is shared with vehicular traffic. | Provides as direct a route as possible along the section. | Provides as direct a route as possible along the section. | Provides as direct a route as possible along the section. |
|  | Coherence | Route does not provide continuity with the rest of the scheme. <br> There is potential for obstructions throughout. | A highly legible route which is well defined and free of obstructions. | A highly legible route which is well defined and free of obstructions. | A highly legible route but may be subject to encroachment and illegal parking by vehicles. |
|  | Attractiveness | Option is not attractive to cyclists. <br> Design does not contribute to the urban design. | Provision is somewhat isolated on northern side of carriageway. | Provides a wellintegrated cycle and pedestrian route. | Provision is not as attractive as segregated cycle tracks. |
|  | Comfort | No provision of cyclist own space. Cyclist may feel uncomfortable interacting with vehicles. | Provides a comfortable facility, suitable for all users. <br> Cyclist progression may be reduced due to necessity of crossing points. | Provides a comfortable facility, suitable for all users. <br> Cyclist progression is maintained throughout the section with minimal crossing points. | Sharing the carriageway with vehicles is less comfortable for many cyclists. <br> Surface is subject to greater levels of damage as it is shared with vehicles. |
|  | Level of service | Level of service is poor. | Separated cyclist and pedestrian channels of travel allows for level of service A. | One way cycle tracks and pedestrian footpaths allow conflicts between modes is minimised. | On road cycle tracks provide a good level of service but may be subject to delays etc due to vehicles in the cycle lane. |


| Context | Criteria | Option 1 | Option 2 | Option 3 | Option 4 |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Conflicts between modes is minimised allowing greater service level. | Raised adjacent tracks allowing greater service level as they provide segregation and avoid delays.. |  |
| Community | Business impact | No business impacts. | No business impacts. | No business impacts. | No business impacts. |
|  | Residential impact | No residential impact. | No residential impact. | No residential impact. | No residential impact. |
|  | Operational impact | Some very minor <br> operational impacts possible due to presence of increased cyclists on road with from adjacent new facilities. | Marginal impact on junctions due to inclusion of new crossing points. <br> Facility will require routine maintenance. | Marginal impact on junctions due to inclusion of new crossing points. <br> Facility will require routine maintenance. | Marginal impact on junctions due to inclusion of new crossing points. <br> Facility will require routine maintenance. |
|  |  |  | Facility will have some impact on surrounding environment during construction. | Facility will have some impact on surrounding environment during construction. | Facility will have some impact on surrounding environment during construction. |
|  | Environmental | No impact to environment. | Existing footpaths requires reconfiguring to allow for two way cycle track by widening into verge on northern side of carriageway. | Existing footpaths requires reconfiguring to allow for one way cycle tracks with removal of grass verges required. | Existing footpaths requires reconfiguring to allow for one way cycle tracks with removal of grass verges required. |
| Delivery | Capital cost | No additional costs to scheme. | Costs associated with kerb relocation and construction of new cycle tracks/footpaths. <br> No carriageway widening required. | Costs associated with kerb relocation and construction of new cycle tracks/footpaths. <br> No carriageway widening required. | Significant cost implications to widening of carriageway on both sides with possibility of utility diversions etc. |
|  | Programme risks | No risks to programme delivery. | Minor risk to programme as a result of unknown services etc but minimised due to construction in verge. | Minor risk to programme as a result of unknown services etc but minimised due to raised adjacent construction | Programme could suffer due to additional difficulties because of widening. |


| Context | Criteria | Option 1 | Option 2 | Option 3 | Option 4 |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Potential <br> conflicts with <br> utilities. |  |
| Ranking |  | $4^{\text {th }}$ | $2^{\text {nd }}$ | $1^{\text {st }}$ | $3^{\text {rd }}$ |

### 6.3.4.6. Link Section 4 - Preferred Option

Option 3 is the preferred option in this location. The provision of new raised adjacent cycle tracks on both sides along the entire section will provide a safe, attractive and accessible route for all road users. It will have no impact on the surrounding businesses and has no impact on any residences while providing a high quality of service. It allows greater access to both sides of the road than Option 2 and reduces the need for crossing points etc. The preferred option is shown below.


Figure 6-19 Link Section 4 - Preferred Option

### 6.3.5. Link Section 5

Section 5 extends from the three arm roundabout on Harry Reynolds Road along Moylaragh Road to the signal controlled junction on the Castlemill Link Road. This section also includes the park lands to the north of Moylaragh Road.

The link road between the Harry Reynolds Road roundabout and the roundabout at Moylaragh Road has an overall width of approximately 8 m . On street parking is provided on the southern side of the carriageway. Footpaths and cycle lanes run adjacent to the carriageway on both sides. A buffer between the footpath and the carriageway is provided through a grass verge. This section of the scheme was a 50 kph speed limit in place.

Moylaragh Road has an overall width of approximately 6.5-7.0m. A footpath runs adjacent to the carriageway on the southern side which is separated from the carriageway by means of a grass verge. The verge is lined with immature trees. There are residences fronting directly onto this road on the southern side along the majority of its length with driveways access across the footpath.

The park land to the north of the road has immature trees planted throughout. A shared path passes through the centre of the park with links to Chieftain's Drive.

Four options were considered for this section and are discussed below.

### 6.3.5.1. Option 1 - Do Nothing.

No provision would be made for cyclists aside from the short sections of existing one-way cycle provision on either side of Moylaragh Road, as indicated in the figure below. Outside of this provision cyclist must share the carriageway with vehicular traffic. Pedestrians would continue to use the existing facilities.


Figure 6-20 Option 1 - Do Nothing

### 6.3.5.2. Option 2 - Cycle Tracks Both Sides with Two Way Segregated Cycle Track Through Park

A two-way segregated cycle track will be provided through the park lands, north of Moylaragh Road. Links from Moylaragh Road will allow pedestrians and cyclist to gain access to the facility. The exact location of this cycle route would be determined during preliminary design in consultation with Fingal County Council's Parks Department. The two-way cycle track would be $3 m$ wide through the park with the existing footpath through the park maintained for pedestrians. The existing one-way cycle tracks on both sides of Moylaragh Road would be upgraded to $2 m$ wide raised adjacent with $2 m$ wide footpaths between Harry Reynolds Road and Chieftain's Drive. The existing roundabout at Chieftain's Drive would be upgraded to a cycle friendly one. The proposed option is shown in the figure below.


Figure 6-21 Option 2 - Two-Way Segregated Cycle Track

### 6.3.5.3. Option 3 - One Way Cycle Tracks Both Sides Along Moylaragh Road

This option includes 2 m wide raised adjacent cycle tracks being provided along both sides of Moylaragh Road between Harry Reynolds Road and the signalised junction at Castlemill Link Road. The existing footpath on the southern side of the road would be widened to $2 m$ with the raised adjacent cycle tracks constructed generally in the existing grass verge. This option would require relocation of the kerb to the southern side of Moylaragh Road and possible widening of the carriageway to maintain acceptable widths for vehicular traffic. The existing roundabout at Chieftain's Drive would be upgraded to a cycle friendly roundabout while the proposed cycle tracks would tie into the existing facilities at the Castemill Link Road. This option is shown in the figure below.


Figure 6-22 Option 3 - One Way Cycle Tracks Both Sides on Moylaragh Road

### 6.3.5.4. Option 4 - Two Way Cycle Track With Shared Street on Chieftain's Drive and Permeability Links in Moylaragh

This option would designate Chieftain's Drive as a shared street, given the character of the road with low speeds and volumes. Designation of the shared space could be achieved with the use of minimal road markings and signage.
Within Moylargah new permeability links would be provided to connect them to the new cycle provisions along the CastleMill Link Road.
The existing roundabout at Chieftain's Drive would be upgraded with a new zebra crossing on the northern arm to allow access to/from the two-way cycle track. A new shared cycle and pedestrian link would be provided between Chieftain's Drive and Castelmilll Link Road. A 3m wide two way segregated cycle track and 2 m wide footpath would be provided on the northern side of Moylaragh Road between Harry Reynolds Road and

Chieftain's Drive. This would require removal of the existing grass verge and possible relocation of the existing kerbline to narrow the carriageway width. This option is shown below.


Figure 6-23 Option 4 - Two Way Cycle Track With Shared Street on Chieftain's Drive

### 6.3.5.5. Option 5 - Two Way Segregated Cycle Track Throughout Section

This option is similar to Option 2 and Option 4 with a 3 m wide two-way segregated cycle tracks and 2 m footpath provided on the northern side of Moylaragh Road between Harry Reynold's Road and Chieftain's Drive. This would require removal of the existing grass verge and possible relocation of the existing kerbline to narrow the carriageway width. The option then continues on to a new two-way cycle track through the park lands between Moylaragh Road and Chieftain's Drive. As in Option 2, links from Moylaragh Road and Chieftain's Road to this cycle track would be provided where possible. A new zebra crossing would be provided on the northern arm only of the roundabout at Chieftain's Drive to allow safe crossing for cyclists and pedestrians. The proposed option is shown below.


Figure 6-24 Option 5- Two Way Segregated Cycle Track Throughout

### 6.3.5.6. Link Section 5 - MCA

The Multi Criteria Analysis and comparison between each option for Link Section 4 is summarised in the table below.

Table 6-7 Link Section 5 - MCA Comparison Matrix

| Context | Criteria | Option 1 | Option 2 | Option 3 | Option 4 | Option 5 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Design | Safety* | Does not provide a safe environment for cyclists as they must share the carriageway with vehicular traffic. | Provides a safe environment for pedestrians and cyclists. <br> The channels of travel are well defined. | Provides a safe environment for pedestrians and cyclists. <br> The channels of travel are well defined. | Provides a designated area for cyclists but also uses a shared street which may reduce safety slightly as users must share space with slow moving vehicles. | Provides a safe environment for pedestrians and cyclists. <br> Segregated cycle way and footpath minimises conflicts between cyclists and pedestrians |
|  | Directness | Does not provide opportunity to overtake as environment is shared with vehicular traffic. | Provides as direct a route as possible along the section. Although the connection to the residential areas on the north and south are not direct. | Provides as direct a route as possible along the section. | Provides as direct a route as possible along the section. | Provides as direct a route as possible along the section. Although the connection to the residential areas on the north and south are not direct.. |
|  | Coherence | Route does not provide continuity with the rest of the scheme. <br> There is potential for obstructions throughout. | A highly legible route which is well defined. <br> Removal of parking on southern side of Moylaragh Road may result in illegal parking blocking the cycle track. | A highly legible route which is well defined. <br> Removal of parking on southern side of Moylaragh Road may result in illegal parking blocking the cycle track along with possibility of parking outside of residences doing the same. | Route is well defined and clear of obstructions throughout | Route is well defined and clear of obstructions throughout. |


| Context | Criteria | Option 1 | Option 2 | Option 3 | Option 4 | Option 5 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Community | Attractiveness | Option is not attractive to cyclists. <br> Design does not contribute to the urban design. | Route is attractive as it is direct and integrates with the existing layout. | Route is attractive as it is direct and integrates with the existing layout.. | Route is attractive as it is direct and integrates with the existing layout.. | Route is generally attractive as it is direct but is isolated to the northern side for a short section. |
|  | Comfort | No provision of cyclist own space cyclist may feel uncomfortabl e interacting with vehicles. | Provides a comfortable facility, suitable for all users. <br> Cyclist progression may be reduced due to necessity of additional crossing points. | Provides a comfortable facility, suitable for all users. <br> Cyclist progression may be reduced due to necessity of additional crossing points. | Provides a comfortable facility, suitable for all users. <br> Cyclist progression is maintained throughout the section with minimal crossing points. | Provides a comfortable facility, suitable for all users. <br> Cyclist progression is maintained throughout the section with minimal crossing points, |
|  | Level of service | Level of service is poor. | Separated cyclist and pedestrian channels of travel allows for level of service A. <br> Conflicts between modes is minimised allowing greater service level. | One way cycle tracks and pedestrian footpaths allow conflicts between modes is minimised. <br> Raised adjacent tracks allowing greater service level as they provide segregation and avoid delays. | Generally provides a high quality of service but is reduced slightly by need to share space with vehicles. | Separated cyclist and pedestrian channels of travel allows for level of service A. <br> Conflicts between modes is minimised allowing greater service level. |
|  | Business impact | No business impacts. | No business impacts. | No business impacts. | No business impacts. | No business impacts. |
|  | Residential impact | No residential impact. | Some impact on residents as parking is removed. | Raised adjacent cycle tracks on southern side of Moylaragh Road directly impact | No residential impact. | No residential impact. |



### 6.3.5.7. Preferred Option - Two Way Cycle Track With Shared Street on Chieftain's Drive and Permeability Links in Moylaragh

Option 4 is the preferred option for this section of the scheme.
This option would designate Chieftain's Drive as a shared street, given the character of the road with low speeds and volumes. Designation of the shared space could be achieved with the use of minimal road markings and signage.
With Moylargah new permeability links would be provided to connect them to the new cycle provisions along the CastleMill Link Road.
The existing roundabout at Chieftain's Drive would be upgraded with a new zebra crossing on the northern arm to allow access to/from the two-way cycle track. A new shared cycle and pedestrian link would be provided between Chieftain's Drive and Castelmilll Link Road. A 3m wide two way segregated cycle track and 2 m wide footpath would be provided on the northern side of Moylaragh Road between Harry Reynolds Road and Chieftain's Drive. This would require removal of the existing grass verge and possible relocation of the existing kerbline to narrow the carriageway width. This option is shown below.


Figure 6-25 Preferred Option Option 4 - Two Way Cycle Track With Shared Street on Chieftain's Drive

### 6.4. Junction Assessments

Junction assessments were carried out on the two junctions highlighted in the figure below. In their current configuration both junctions operate as roundabouts and they are described in detail in the following sections.


Figure 6-26 Junctions for Assessment

### 6.4.1. Junction 1

Junction 1 comprises of a four arm roundabout. The L1390 enters the roundabout from the west with two entry lanes. Hamilton Road (L5460) enters the roundabout from the east with two entry lanes. Dublin Street enters the roundabout from both north and south directions. Entries into the roundabout are by way of two entry lanes while all exits from the roundabout are done so using wide single lane exits. Entry and exit widths vary from $6.0-8.0 \mathrm{~m}$. The roundabout generally operates as a two lane circulating carriageway. The inscribed circle diameter of the roundabout is approximately 50 m .

Cyclists are catered for via one way cycle lanes on the Hamilton Road and L1390 while a two-way facility ends close to the Dublin Street South exit.

There are uncontrolled crossing points at the junction entry/exit on L1390 and Dublin Street South while there is a controlled pedestrian crossing on Hamilton Road. There is a controlled signal crossing for pedestrians on Dublin Street North approximately 60 m north of the junction. The figure below shows the existing layout.


Figure 6-27 Existing Layout of Junction 1

### 6.4.1.1. Option 1 - Zebra Crossing on Dublin Street North

The current configuration would generally be maintained. A new zebra crossing would be provided close to the entry/exit of Dublin Street North with a 3 m segregated two-way cycle track and 2 m footpath provided on both sides. This would require the existing geometry of this arm to be modified including relocation of kerbs and reducing of entry/exit widths and radii as shown in the sketch in the below figure.


Figure 6-28 Option 1 - Zebra Crossing On Dublin Street North

### 6.4.1.2. Option 2 - Cycle Friendly Roundabout

The current roundabout layout will be reconfigured to have single lane entry and exits on all arms and entry and exit widths reduced 3.5 m in width. The inscribed circle diameter of the roundabout would also be reduced to approximately 30 m . This would result in a cycle friendly roundabout in line with the National Cycle Manual. 4 m wide zebra crossing facilities would be provided on the Dublin Street North and Hamilton Road arms. The figure below shows the proposed roundabout reconfiguration.


Figure 6-29 Option 2 - Cycle Friendly Roundabout

### 6.4.1.3. Option 3 - Signalised Junction

The current roundabout layout would be reconfigured to a signalised junction. All entry lanes to the junction are provided by 3.5 m wide lanes. All exits from the junction are single exit lanes of 3.5 m in width.

Entry to the junction from Dublin Street North will be provided by two lanes, a right turn lane and a straight ahead/left turn lane.

Entry to the junction from Hamilton Road will be provided by three lanes, a right turn, straight ahead and left turn. The left turn lane is provided through the inclusion of a left turn pocket.

Entry to the junction along Dublin Street South is provided through two lanes, a right turn and straight ahead. A left turn slip onto the L1390 is provided in advance of the junction.

Entry to the junction along the L1390 is provided through three lanes, a right turn, straight ahead and left turn lane. The left turn lane is provided through the inclusion of a left turn pocket.

The figure below shows a possible configuration of the signalised junction.


Figure 6-30 Option 3 - Signalised Junction

### 6.4.1.4. Junction 1 - MCA

The Multi Criteria Analysis and comparison between each option for Junction 1 is summarised in the table below. As part of the analysis, the capacity of each of the junction arrangements was assessed using Junctions 9 and Linsig as appropriate. A summary of the results of this analysis are included in Appendix 1.

Table 6-8 Junction 1 - MCA Comparison Matrix

| Context | Criteria | Option 1 | Option 2 | Option 3 |
| :---: | :---: | :---: | :---: | :---: |
| Design | Safety* | Layout provides safe controlled crossing points for vulnerable road users accessing the scheme. <br> Zebra crossings have marginally less control than traffic signals. | Layout provides safe controlled crossing points for vulnerable road users accessing the scheme. <br> Zebra crossings have marginally less control than traffic signals. | Layout provides safe controlled crossing points for vulnerable road users accessing the scheme. |
|  | Directness | Provides a direct route that is very slightly set back from the desire line. | Provides as direct a route as possible across the junction. | Provides as direct a route as possible across the junction. |
|  | Coherence | Layout is legible and continuous with existing and proposed facilities. | Layout is legible and continuous with existing and proposed facilities. | Layout is legible and continuous with existing and proposed facilities. |
|  | Attractiveness | Provision of controlled crossings and segregated cycle and pedestrian facilities makes layout attractive to | Provision of controlled crossings and segregated cycle and pedestrian facilities makes layout attractive to | Provision of controlled crossings and segregated cycle and pedestrian facilities makes layout attractive to |



### 6.4.1.5. Junction 1 - Preferred Option

The preferred option for this junction is Option 1. This option would realign Dublin Street North slightly on approach to the roundabout to enable provision of a 3 m wide two-way cycle track and 2 m footpath on both
sides with a zebra crossing provided across the road. This junction provides a good level of service for vulnerable users while not unduly affecting the capacity of the existing roundabout.

However, it is acknowledged due to forecast increases in traffic flows within Balbriggan and for other traffic management reasons the end solution for this junction will involve upgrading this junction to a signalised junction.

The signalised junction option will be progressed within the next stage of the project and the signalised junction option may be part of the Part 8 scheme.

### 6.4.2. Junction 2

Junction 2 comprises of a three arm roundabout. Moylaragh Road enters the roundabout from the west with two entry lanes. Harry Reynolds Road enters the roundabout from both east and south directions. All entries and exits of the roundabout are done so using single lanes. Entry and exit widths vary from 4.0-5.0m. The inscribed circle diameter of the roundabout is approximately 32 m .

Pedestrians and cyclist are catered for via one way cycle lanes and segregated footpaths along Moylaragh Road. Harry Reynolds Road has no cycle provision, pedestrians are catered for via the existing footpaths on both sides of the carriageway.

Uncontrolled crossing facilities are available on the Moylaragh Road entry and the Harry Reynolds South entry.
The figure below shows the existing junction configuration.


Figure 6-31 Junction 2 - Existing Layout

### 6.4.2.1. Option 1 - Cycle Friendly Roundabout

This option would alter the existing roundabout geometry to provide a cycle friendly roundabout in accordance with the National Cycle Manual. This would include reducing of entry and exit widths and radii and provision of new cycle and pedestrian facilities around the entire roundabout with zebra crossings provided across each arm. The proposed layout is shown in the figure below.


Figure 6-32 Option 1 - Cycle Friendly Roundabout

### 6.4.2.2. Option 2 - Signal Controlled Junction

The current roundabout layout would be reconfigured to a signalised junction. All entry lanes to the junction will be provided by 3.5 m wide lanes while all exits from the junction are single exit lanes of 3.5 m in width. Toucan crossings would be provided on all arms of the junction. The figure below shows the proposed layout.


Figure 6-33 Option 2 - Signal Controlled Junction

### 6.4.2.3. Junction 2 - MCA

The Multi Criteria Analysis and comparison between each option for Junction 2 is summarised in the table below. As part of the analysis, the capacity of each of the junction arrangements was assessed using Junctions 9 and Linsig as appropriate. A summary of the results of this analysis are included in Appendix 1.

Table 1. MCA Performance Matrix

| Context | Criteria | Option 1 | Option 2 |
| :---: | :---: | :---: | :---: |
| Design | Safety* | Layout provides safe controlled crossing points for vulnerable road users accessing the scheme. <br> Zebra crossings have marginally less control than traffic signals. | Layout provides safe controlled crossing points for vulnerable road users accessing the scheme. |
|  | Directness | Provides as direct a route as possible across the junction. | Provides as direct a route as possible across the junction. |
|  | Coherence | Layout is legible and continuous with proposed facilities. | Layout is legible and continuous with proposed facilities. |
|  | Attractiveness | Provision of controlled crossings and segregated cycle and pedestrian facilities makes layout attractive to vulnerable road users. | Provision of controlled crossings and segregated cycle and pedestrian facilities makes layout attractive to vulnerable road users. |
|  | Comfort | Tighter roundabout geometry and reduced vehicle speeds ensure zebra crossing offer a comfortable and safe crossing point. | Controlled crossing points and controlled traffic movements makes junction comfortable for all users. |
|  | Level of service | Provides a high quality level of service for cyclists/pedestrians. | Provides a high quality level of service for cyclists/pedestrians <br> Some delays possible at signal controlled crossings as opposed to zebras. |
| Community | Business impact | Minimal impacts to businesses. | Minimal impacts to businesses. |
|  | Residential impact | Minimal impacts to residents. | Minimal impacts to residents. |
|  | Operational impact | Junction operates within capacity without unduly affecting vehicular traffic. | Junction operates within capacity without unduly affecting vehicular traffic. |
|  | Environmental | Some environmental impact associated with removal of grass verges etc. | Some environmental impact associated with removal of grass verges etc. |


| Context | Criteria | Option 1 | Option 2 |
| :---: | :---: | :---: | :---: |
| Delivery | Capitol cost | Additional costs to scheme for reconfiguration works including kerb relocations etc. | Higher additional costs to reconfigure junction and install new traffic signals for entire junction. |
|  | Programme risks | Programme could suffer due to extent of works required to reconfigure the junction layout. | Programme could suffer due to extent of works required to reconfigure the junction layout. |
| Ranking |  | 1st | $2^{\text {nd }}$ |

### 6.4.2.4. Junction 2 - Preferred Option

Option 1 is the preferred option for Junction 2. The reconfiguration of the junction to a cycle friendly roundabout ensures that cyclists and pedestrians have easy, segregated access around the entire roundabout and can cross at zebra crossings without any delay. The junction will still operate within capacity and vehicle users will not be unduly delayed.


Figure 6-34 Junction 2 - Preferred Option

## 7. Preferred Route Option

### 7.1. Description

The Preferred Route for the scheme is shown in the figure below.


Figure 7-1 Preferred Route

## It includes:

- Permeability improvements in Moylaragh
- New zebra crossing across Chieftain's Drive at the roundabout
- New two-way cycle track on northern side of Moylaragh Road between Chieftain's Drive and Harry Reynolds Road
- Existing roundabout at Harry Reynolds Road/Moylaragh Road to be reconfigured to cycle friendly roundabout
- New two-way raised cycle track on Harry Reynolds Road between roundabout and Drogheda Street
- New one-way raised adjacent cycle tracks on Harry Reynolds Road between roundabout and Chapel Street junction
- Existing signal controlled junction at Chapel Street to be maintained as is
- New one-way raised adjacent cycle tracks on Harry Reynolds Road between Chapel Street junction to just north of junction with public car park entrance
- New toucan crossing on Harry Reynolds Road at change between one-way and two-way cycle tracks
- New two-way cycle track adjacent to public car park exit road and beside existing car park boundary wall
- New two-way cycle tracks around Dublin Street North arm of roundabout at Dublin Street/L1390/Hamilton Road
- New zebra crossing at Dublin Street North arm (although the signalised junction may be delivered, due to traffic management reasons
- Existing cycle tracks on Hamilton Road to be maintained
- New toucan crossing on Hamilton Road near entrance to Town Park.
- Provision of two way cycle track between the new toucan crossing and the school entrance.
- Environmental Improvements to the laneway to Curran Park and to the schools on Hamilton Road.
- New cycle track to be provided through park along Bracken River - exact route to be determined during preliminary design

The preferred route drawing is shown in Appendix D .

### 7.2. Preliminary Cost Estimate

A preliminary cost estimate for the scheme has been prepared using the rates from recent urban road schemes. A contingency of $20 \%$ has been allocated to the overall cost to allow for any unforeseen items. A breakdown cost estimate of the scheme is shown in Table 7-1. At a feasibility stage, the cost estimate would be in the order of $+/-50 \%$.

The provision of the signalised junction at the junction of Dublin St / Harry Reynolds Road / Hamilton Road would add an additional $€ 1,000,000$ to the overall cost estimate.

## Table 7-1 Cost estimate per section

| Section | Cost Estimate |
| :--- | :--- |
| Section 1 | $€ 100,000$ |
| Section 2 | $€ 35,000$ |
| Section 3 | $€ 1,000,000$ |
| Section 4 | $€ 180,000$ |
| Section 5 | $€ 290,000$ |
| Junction 1 | $€ 250,000$ (Zebra Crossing) |
| Junction 2 | $€ 750,000$ |
| Upgrade of Bridges | $€ 100,000$ |
| Works within The Park | $€ 200,000$ |
| Environmental Improvements and | $€ 80,000$ |
| Permeability Links | $€ 530,000$ |
| Contingency | $€ 3,515,000$ |
| Total |  |

The provision of the signalised junction at the junction of Dublin St / Harry Reynolds Road / Hamilton Road would add an additional $€ 1,000,000$ to the overall cost estimate.

Appendices

## Appendix A. Junction Analysis

## Junctions 9



Filename: R132 Roundabout with Zebra Crossing on Dublin Street.j9
Path: U:\5165984\7 Calcs\72Model
Report generation date: 17/10/2018 11:05:57
"2018, AM
„2018, PM

## Summary of junction performance

|  | AM |  |  |  |  | PM |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Queue (PCU) | Delay (s) | RFC | LOS | Queue (PCU) | Delay (s) | RFC | LOS |  |  |
|  | $\mathbf{2 0 1 8}$ |  |  |  |  |  |  |  |  |  |
| Arm 1 | 2.1 | 14.15 | 0.68 | B | 1.6 | 10.34 | 0.62 | B |  |  |
| Arm 2 | 1.1 | 6.90 | 0.52 | A | 0.6 | 5.30 | 0.37 | A |  |  |
| Arm 3 | 0.8 | 5.56 | 0.46 | A | 0.8 | 5.48 | 0.44 | A |  |  |
| Arm 4 | 0.6 | 3.43 | 0.38 | A | 0.4 | 2.91 | 0.31 | A |  |  |

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.
Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle.

## File summary

File Description

| Title | (untitled) |
| :--- | :--- |
| Location |  |
| Site number |  |
| Date | $30 / 04 / 2018$ |
| Version |  |
| Status | (new file) |
| Identifier |  |
| Client |  |
| Jobnumber |  |
| Enumerator | ATKINSMCCARTHY\MCollins |
| Description |  |

## Units

| Distance units | Speed units | Traffic units input | Traffic units results | Flow units | Average delay units | Total delay units | Rate of delay units |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| m | kph | PCU | PCU | perHour | s | perMin |  |

## Analysis Options

| Calculate Queue Percentiles | Calculate residual capacity | RFC Threshold | Average Delay threshold (s) | Queue threshold (PCU) |
| :--- | :---: | :---: | :---: | :---: |
|  |  | 0.85 | 36.00 | 20.00 |

## Demand Set Summary

| ID | Scenario name | Time Period name | Traffic profile type | Start time (HH:mm) | Finish time (HH:mm) | Time segment length (min) |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: |
| D1 | 2018 | AM | ONE HOUR | $08: 00$ | $09: 30$ | 15 |
| D2 | 2018 | PM | ONE HOUR | $17: 00$ | $18: 30$ | 15 |

## Analysis Set Details

| ID | Network flow scaling factor (\%) |
| :---: | :---: |
| A1 | 100.000 |

## 2018, AM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Vehicle Mix | HV\% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in <br> PCUs or Vehs. |  |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | 7.32 | A |

## Junction Network Options

| Driving side | Lighting |
| :---: | :---: |
| Left | Normal/unknown |

## Arms

## Arms

| Arm | Name | Description |
| :---: | :---: | :---: |
| $\mathbf{1}$ | untitled |  |
| $\mathbf{2}$ | untitled |  |
| $\mathbf{3}$ | untitled |  |
| $\mathbf{4}$ | untitled |  |

## Roundabout Geometry

| Arm | V-Approach road half- <br> width $(\mathbf{m})$ | E-Entry width <br> $(\mathbf{m})$ | I' - Effective flare <br> length $(\mathbf{m})$ | R - Entry radius <br> $(\mathbf{m})$ | D - Inscribed circle <br> diameter (m) | PHI - Conflict (entry) <br> angle (deg) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 3.00 | 5.00 | 5.0 | 20.0 | 51.0 |  |
| $\mathbf{2}$ | 3.80 | 7.30 | 6.5 | 22.0 | 50.0 |  |
| $\mathbf{3}$ | 3.30 | 7.30 | 11.5 | 24.0 | 51.0 |  |
| $\mathbf{4}$ | 5.90 | 7.90 | 19.5 | 28.0 | 51.0 |  |

## Zebra Crossings

| Arm | Space between crossing and junction entry (Zebra) (PCU) | Vehicles queueing on exit (Zebra) (PCU) | Central Refuge | Crossing data type | Crossing length (m) | Crossing time (s) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1.50 | 1.50 |  | Distance | 6.00 | 4.29 |

## Pelican/Puffin Crossings

| Arm | Space between crossing and <br> junction entry (Signalised) (PCU) | Amber time <br> preceding red (s) | Amber time <br> regarded as green <br> (s) | Time from traffic red <br> start to green man start <br> $\mathbf{( s )}$ | Time period <br> green man shown <br> (s) | Clearance <br> Period (s) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{2}$ | 3.50 | 3.00 | 2.90 | 2.00 | 6.00 |  |

## Slope / Intercept / Capacity

Roundabout Slope and Intercept used in model

| Arm | Final slope | Final intercept (PCU/hr) |
| :---: | :---: | :---: |
| $\mathbf{1}$ | 0.470 | 1093 |
| $\mathbf{2}$ | 0.521 | 1398 |
| $\mathbf{3}$ | 0.539 | 1461 |
| $\mathbf{4}$ | 0.655 | 2081 |

[^0]
## Traffic Demand

## Demand Set Details

| ID | Scenario name | Time Period name | Traffic profile type | Start time (HH:mm) | Finish time (HH:mm) | Time segment length (min) |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: |
| D1 | 2018 | AM | ONE HOUR | $08: 00$ | $09: 30$ | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

## Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Average Demand (PCU/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | $\checkmark$ | 490 | 100.000 |
| $\mathbf{2}$ |  | $\checkmark$ | 511 | 100.000 |
| $\mathbf{3}$ |  | $\checkmark$ | 492 | 100.000 |
| $\mathbf{4}$ |  | $\checkmark$ | 580 | 100.000 |

## Demand overview (Pedestrians)

| Arm | Average pedestrian flow (Ped/hr) |
| :---: | :---: |
| $\mathbf{1}$ | 100.00 |
| $\mathbf{2}$ | 20.00 |
| $\mathbf{3}$ |  |
| $\mathbf{4}$ |  |

## Origin-Destination Data

Demand (PCU/hr)

|  | To |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ |
|  | $\mathbf{1}$ | 0 | 115 | 245 | 130 |
|  | $\mathbf{2}$ | 139 | 0 | 183 | 189 |
|  | $\mathbf{3}$ | 184 | 151 | 0 | 157 |
|  | $\mathbf{4}$ | 154 | 305 | 121 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ |
|  | $\mathbf{1}$ | 0 | 0 | 0 | 0 |
|  | $\mathbf{2}$ | 0 | 0 | 0 | 0 |
|  | $\mathbf{3}$ | 0 | 0 | 0 | 0 |
|  | $\mathbf{4}$ | 0 | 0 | 0 | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (PCU) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.68 | 14.15 | 2.1 | B |
| $\mathbf{2}$ | 0.52 | 6.90 | 1.1 | A |
| $\mathbf{3}$ | 0.46 | 5.56 | 0.8 | A |
| $\mathbf{4}$ | 0.38 | 3.43 | 0.6 | A |

## Main Results for each time segment

08:00-08:15

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity <br> (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 369 | 433 | 75.29 | 889 | 0.415 | 366 | 0.7 | 6.852 | A |
| 2 | 385 | 371 | 15.06 | 1170 | 0.329 | 383 | 0.5 | 4.564 | A |
| 3 | 370 | 343 |  | 1276 | 0.290 | 369 | 0.4 | 3.960 | A |
| 4 | 437 | 355 |  | 1829 | 0.239 | 435 | 0.3 | 2.580 | A |

08:15-08:30

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 440 | 518 | 89.90 | 849 | 0.519 | 439 | 1.1 | 8.760 | A |
| 2 | 459 | 445 | 17.98 | 1132 | 0.406 | 459 | 0.7 | 5.340 | A |
| 3 | 442 | 411 |  | 1239 | 0.357 | 442 | 0.6 | 4.510 | A |
| 4 | 521 | 426 |  | 1772 | 0.294 | 521 | 0.4 | 2.877 | A |

08:30-08:45

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 540 | 634 | 110.10 | 794 | 0.680 | 536 | 2.0 | 13.748 | B |
| 2 | 563 | 543 | 22.02 | 1082 | 0.520 | 561 | 1.1 | 6.891 | A |
| 3 | 542 | 502 |  | 1190 | 0.455 | 541 | 0.8 | 5.533 | A |
| 4 | 639 | 521 |  | 1689 | 0.378 | 638 | 0.6 | 3.424 | A |

08:45-09:00

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 540 | 635 | 110.10 | 793 | 0.680 | 539 | 2.1 | 14.150 | B |
| 2 | 563 | 546 | 22.02 | 1084 | 0.519 | 563 | 1.1 | 6.899 | A |
| 3 | 542 | 504 |  | 1189 | 0.456 | 542 | 0.8 | 5.560 | A |
| 4 | 639 | 522 |  | 1688 | 0.378 | 639 | 0.6 | 3.430 | A |

09:00-09:15

| Arm | Total Demand <br> $(\mathbf{P C U} / \mathbf{h r})$ | Circulating flow <br> $(\mathbf{P C U} / \mathrm{hr})$ | Pedestrian <br> demand <br> $(\mathbf{P e d} / \mathbf{h r})$ | Capacity <br> $(\mathbf{P C U} / \mathbf{h r})$ | RFC | Throughput <br> $(\mathbf{P C U} / \mathbf{h r})$ | End queue <br> $(\mathbf{P C C U})$ | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 440 | 520 | 89.90 | 848 | 0.520 | 444 | 1.1 | 9.007 | A |
| $\mathbf{2}$ | 459 | 449 | 17.98 | 1135 | 0.405 | 461 | 0.7 | 5.355 | A |
| $\mathbf{3}$ | 442 | 414 |  | 1238 | 0.357 | 443 | 0.6 | 4.539 | A |
| $\mathbf{4}$ | 521 | 427 |  | 1771 | 0.294 | 522 | 0.4 | 2.884 | A |

09:15-09:30

| Arm | Total Demand <br> $(\mathbf{P C U} / \mathbf{h r})$ | Circulating flow <br> $(\mathbf{P C U} / \mathbf{h r})$ | Pedestrian <br> demand <br> $(\mathbf{P e d} / \mathbf{h r})$ | Capacity <br> $(\mathbf{P C U} / \mathbf{h r})$ | RFC | Throughput <br> $(\mathbf{P C U} / \mathbf{h r )}$ | End queue <br> $(\mathbf{P C U})$ | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 369 | 435 | 75.29 | 888 | 0.415 | 370 | 0.7 | 6.975 |  |
| $\mathbf{2}$ | 385 | 375 | 15.06 | 1173 | 0.328 | 385 | 0.5 | 4.577 | A |
| $\mathbf{3}$ | 370 | 346 |  | 1274 | 0.291 | 371 | 0.4 | 3.986 | A |
| $\mathbf{4}$ | 437 | 357 |  | 1828 | 0.239 | 437 | 0.3 | 2.591 | A |

## 2018, PM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Pedestrian Crossing | Arm 1 - Pedestrian <br> crossing | Pedestrian crossing uses default flow of 0. Is this correct? |
| Warning | Pedestrian Crossing | Arm 2 - Pedestrian <br> crossing | Pedestrian crossing uses default flow of 0. Is this correct? |
| Warning | Vehicle Mix |  | HV\% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in <br> PCUs or Vehs. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | 6.09 | A |

## Junction Network Options

| Driving side | Lighting |
| :---: | :---: |
| Left | Normal/unknown |

## Traffic Demand

## Demand Set Details

| ID | Scenario name | Time Period name | Traffic profile type | Start time (HH:mm) | Finish time (HH:mm) | Time segment length (min) |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: |
| D2 | 2018 | PM | ONE HOUR | $17: 00$ | $18: 30$ | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

## Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Average Demand (PCU/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | $\checkmark$ | 505 | 100.000 |
| $\mathbf{2}$ |  | $\checkmark$ | 366 | 100.000 |
| $\mathbf{3}$ |  | $\checkmark$ | 466 | 100.000 |
| $\mathbf{4}$ |  | $\checkmark$ | 495 | 100.000 |

Demand overview (Pedestrians)

| Arm | Average pedestrian flow (Ped/hr) |
| :---: | :---: |
| $\mathbf{1}$ | 0.00 |
| 2 | 0.00 |
| 3 |  |
| 4 |  |

## Origin-Destination Data

Demand (PCU/hr)

|  | To |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ |
|  | $\mathbf{1}$ | 0 | 75 | 228 | 202 |
|  | $\mathbf{2}$ | 99 | 0 | 77 | 190 |
|  | $\mathbf{3}$ | 238 | 79 | 0 | 149 |
|  | $\mathbf{4}$ | 208 | 167 | 120 | $\mathbf{0}$ |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ |
|  | $\mathbf{1}$ | 0 | 0 | 0 | 0 |
|  | $\mathbf{2}$ | 0 | 0 | 0 | 0 |
|  | $\mathbf{3}$ | 0 | 0 | 0 | 0 |
|  | $\mathbf{4}$ | $\mathbf{0}$ | 0 | 0 | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (PCU) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.62 | 10.34 | 1.6 | B |
| $\mathbf{2}$ | 0.37 | 5.30 | 0.6 | A |
| $\mathbf{3}$ | 0.44 | 5.48 | 0.8 | A |
| $\mathbf{4}$ | 0.31 | 2.91 | 0.4 | A |

## Main Results for each time segment

17:00-17:15

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 380 | 275 | 0.00 | 964 | 0.394 | 378 | 0.6 | 6.112 | A |
| 2 | 276 | 412 | 0.00 | 1184 | 0.233 | 274 | 0.3 | 3.955 | A |
| 3 | 351 | 368 |  | 1263 | 0.278 | 349 | 0.4 | 3.935 | A |
| 4 | 373 | 312 |  | 1876 | 0.199 | 372 | 0.2 | 2.391 | A |

17:15-17:30

| Arm | Total Demand <br> (PCU/hr) | Circulating flow <br> $(\mathbf{P C U} / \mathbf{h r})$ | Pedestrian <br> demand <br> $(\mathbf{P e d} / \mathbf{h r})$ | Capacity <br> $(\mathbf{P C U} / \mathbf{h r})$ | RFC | Throughput <br> $(\mathbf{P C U} / \mathbf{h r})$ | End queue <br> $(\mathbf{P C U})$ | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 454 | 329 | 0.00 | 939 | 0.484 | 453 | 0.9 | 7.395 | A |
| $\mathbf{2}$ | 329 | 493 | 0.00 | 1141 | 0.288 | 329 | 0.4 | 4.430 | A |
| $\mathbf{3}$ | 419 | 441 |  | 1223 | 0.342 | 418 | 0.5 | 4.469 | A |
| $\mathbf{4}$ | 445 | 374 |  | 1836 | 0.242 | 445 | 0.3 | 2.587 | A |

17:30-17:45

| Arm | Total Demand <br> $(\mathbf{P C U} / \mathbf{h r})$ | Circulating flow <br> $(\mathbf{P C U} / \mathbf{h r})$ | Pedestrian <br> demand <br> $(\mathbf{P e d} / \mathbf{h r})$ | Capacity <br> $(\mathbf{P C U} / \mathbf{h r})$ | $\mathbf{R F C}$ | Throughput <br> $(\mathbf{P C U} / \mathbf{h r})$ | End queue <br> $(\mathbf{P C U})$ | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 556 | 403 | 0.00 | 904 | 0.615 | 553 | 1.6 | 10.195 |  |
| $\mathbf{2}$ | 403 | 603 | 0.00 | 1084 | 0.372 | 402 | 0.6 | 5.277 | A |
| $\mathbf{3}$ | 513 | 539 |  | 1170 | 0.438 | 512 | 0.8 | 5.451 | A |
| $\mathbf{4}$ | 545 | 457 |  | 1781 | 0.306 | 545 | 0.4 | 2.909 | A |

17:45-18:00

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 556 | 403 | 0.00 | 904 | 0.615 | 556 | 1.6 | 10.342 | B |
| 2 | 403 | 605 | 0.00 | 1083 | 0.372 | 403 | 0.6 | 5.296 | A |
| 3 | 513 | 541 |  | 1170 | 0.439 | 513 | 0.8 | 5.483 | A |
| 4 | 545 | 458 |  | 1781 | 0.306 | 545 | 0.4 | 2.912 | A |

18:00-18:15

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity <br> (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 454 | 329 | 0.00 | 938 | 0.484 | 456 | 1.0 | 7.511 | A |
| 2 | 329 | 497 | 0.00 | 1139 | 0.289 | 330 | 0.4 | 4.452 | A |
| 3 | 419 | 443 |  | 1222 | 0.343 | 420 | 0.5 | 4.494 | A |
| 4 | 445 | 375 |  | 1835 | 0.242 | 445 | 0.3 | 2.592 | A |

18:15-18:30

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 380 | 276 | 0.00 | 964 | 0.395 | 381 | 0.7 | 6.197 | A |
| 2 | 276 | 415 | 0.00 | 1182 | 0.233 | 276 | 0.3 | 3.977 | A |
| 3 | 351 | 370 |  | 1261 | 0.278 | 351 | 0.4 | 3.959 | A |
| 4 | 373 | 314 |  | 1875 | 0.199 | 373 | 0.2 | 2.398 | A |

## Junctions 9



Filename: R132 Reconfigured to Cycle Friendly Roundabout.j9
Path: U:\5165984\7 Calcs\72Model
Report generation date: 17/10/2018 11:04:44
"2018, AM
„2018, PM
Summary of junction performance

|  | AM |  |  |  |  | PM |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Queue (PCU) | Delay (s) | RFC | LOS | Queue (PCU) | Delay (s) | RFC | LOS |  |  |  |
|  | $\mathbf{2 0 1 8}$ |  |  |  |  |  |  |  |  |  |  |
| Arm 1 | 9.2 | 65.34 | 0.93 | F | 5.1 | 34.99 | 0.85 | D |  |  |  |
| Arm 2 | 8.5 | 58.40 | 0.92 | F | 2.5 | 22.76 | 0.72 | C |  |  |  |
| Arm 3 | 5.7 | 40.20 | 0.87 | E | 7.2 | 53.99 | 0.90 | F |  |  |  |
| Arm 4 | 24.2 | 132.97 | 1.04 | F | 7.4 | 52.75 | 0.91 | F |  |  |  |

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.
Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle.

## File summary

File Description

| Title | (untitled) |
| :--- | :--- |
| Location |  |
| Site number |  |
| Date | $09 / 05 / 2018$ |
| Version |  |
| Status | (new file) |
| Identifier |  |
| Client |  |
| Jobnumber |  |
| Enumerator | ATKINSMCCARTHYMCollins |
| Description |  |

## Units

| Distance units | Speed units | Traffic units input | Traffic units results | Flow units | Average delay units | Total delay units | Rate of delay units |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| m | kph | PCU | PCU | perHour | s | -Min | perMin |

## Analysis Options

| Calculate Queue Percentiles | Calculate residual capacity | RFC Threshold | Average Delay threshold (s) | Queue threshold (PCU) |
| :--- | :---: | :---: | :---: | :---: |
|  |  | 0.85 | 36.00 | 20.00 |

## Demand Set Summary

| ID | Scenario name | Time Period name | Traffic profile type | Start time (HH:mm) | Finish time (HH:mm) | Time segment length (min) |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: |
| D1 | 2018 | AM | ONE HOUR | $08: 00$ | $09: 30$ | 15 |
| D2 | 2018 | PM | ONE HOUR | $17: 00$ | $18: 30$ | 15 |

## Analysis Set Details

| ID | Network flow scaling factor (\%) |
| :---: | :---: |
| A1 | 100.000 |

## 2018, AM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Pedestrian Crossing | Arm 1 - Pedestrian <br> crossing | Pedestrian crossing uses default flow of 0. Is this correct? |
| Warning | Pedestrian Crossing | Arm 2 - Pedestrian <br> crossing | Pedestrian crossing uses default flow of 0. Is this correct? |
| Warning | Pedestrian Crossing | Arm 3 - Pedestrian <br> crossing | Pedestrian crossing uses default flow of 0. Is this correct? |
| Warning | Pedestrian Crossing | Arm 4 - Pedestrian <br> crossing | Pedestrian crossing uses default flow of 0. Is this correct? |
| Warning | Vehicle Mix |  | HV\% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in <br> PCUs or Vehs. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | 76.58 | F |

## Junction Network Options

| Driving side | Lighting |
| :---: | :---: |
| Left | Normal/unknown |

## Arms

## Arms

| Arm | Name | Description |
| :---: | :---: | :--- |
| $\mathbf{1}$ | untitled |  |
| $\mathbf{2}$ | untitled |  |
| $\mathbf{3}$ | untitled |  |
| 4 | untitled |  |

Roundabout Geometry

| Arm | V - Approach road half- <br> width (m) | E - Entry width <br> $(\mathbf{m})$ | I' - Effective flare <br> length (m) | R - Entry radius <br> $(\mathbf{m})$ | D - Inscribed circle <br> diameter (m) | PHI - Conflict (entry) <br> angle (deg) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 3.00 | 3.50 | 5.0 | 10.0 | 35.0 |  |
| $\mathbf{2}$ | 3.00 | 3.50 | 5.0 | 10.0 | 35.0 |  |
| $\mathbf{3}$ | 3.00 | 3.50 | 5.0 | 10.0 | 30.0 |  |
| $\mathbf{4}$ | 3.00 | 3.50 | 5.0 | 10.0 | 70.0 |  |

## Zebra Crossings

| Arm | Space between crossing and junction entry (Zebra) (PCU) | Vehicles queueing on exit (Zebra) (PCU) | Central Refuge | Crossing data type | Crossing length (m) | Crossing time (s) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1.00 | 1.00 |  | Distance | 6.00 | 4.29 |
| 2 | 1.00 | 1.00 |  | Distance | 6.00 | 4.29 |
| 3 | 1.00 | 1.00 |  | Distance | 6.00 | 4.29 |
| 4 | 1.00 | 1.00 |  | Distance | 6.00 | 4.29 |

## Slope / Intercept / Capacity

Roundabout Slope and Intercept used in model

| Arm | Final slope | Final intercept (PCU/hr) |
| :---: | :---: | :---: |
| $\mathbf{1}$ | 0.418 | 832 |
| $\mathbf{2}$ | 0.418 | 832 |
| $\mathbf{3}$ | 0.418 | 832 |
| $\mathbf{4}$ | 0.418 | 832 |

The slope and intercept shown above include any corrections and adjustments.

## Traffic Demand

## Demand Set Details

| ID | Scenario name | Time Period name | Traffic profile type | Start time (HH:mm) | Finish time (HH:mm) | Time segment length (min) |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: |
| D1 | 2018 | AM | ONE HOUR | $08: 00$ | $09: 30$ | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

## Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Average Demand (PCU/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | $\checkmark$ | 490 | 100.000 |
| $\mathbf{2}$ |  | $\checkmark$ | 511 | 100.000 |
| $\mathbf{3}$ |  | $\checkmark$ | 492 | 100.000 |
| $\mathbf{4}$ |  | $\checkmark$ | 580 | 100.000 |

## Demand overview (Pedestrians)

| Arm | Average pedestrian flow (Ped/hr) |
| :---: | :---: |
| $\mathbf{1}$ | 0.00 |
| 2 | 0.00 |
| 3 | 0.00 |
| 4 | 0.00 |

## Origin-Destination Data

Demand (PCU/hr)

|  | To |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ |
|  | $\mathbf{1}$ | 0 | 115 | 245 | 130 |
|  | $\mathbf{2}$ | 139 | 0 | 183 | 189 |
|  | $\mathbf{3}$ | 184 | 151 | 0 | 157 |
|  | $\mathbf{4}$ | 154 | 305 | 121 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ |
|  | $\mathbf{1}$ | 0 | 0 | 0 | 0 |
|  | $\mathbf{2}$ | 0 | 0 | 0 | 0 |
|  | $\mathbf{3}$ | 0 | 0 | 0 | 0 |
|  | $\mathbf{4}$ | 0 | 0 | 0 | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (PCU) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.93 | 65.34 | 9.2 | F |
| $\mathbf{2}$ | 0.92 | 58.40 | 8.5 | F |
| $\mathbf{3}$ | 0.87 | 40.20 | 5.7 | E |
| $\mathbf{4}$ | 1.04 | 132.97 | 24.2 | F |

## Main Results for each time segment

08:00-08:15

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 369 | 428 | 0.00 | 653 | 0.565 | 364 | 1.3 | 12.262 | B |
| 2 | 385 | 368 | 0.00 | 678 | 0.568 | 380 | 1.3 | 11.883 | B |
| 3 | 370 | 340 | 0.00 | 689 | 0.537 | 366 | 1.1 | 10.981 | B |
| 4 | 437 | 352 | 0.00 | 684 | 0.638 | 430 | 1.7 | 13.810 | B |

08:15-08:30

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity <br> (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 440 | 513 | 0.00 | 617 | 0.713 | 436 | 2.3 | 19.410 | C |
| 2 | 459 | 441 | 0.00 | 647 | 0.710 | 455 | 2.3 | 18.360 | C |
| 3 | 442 | 408 | 0.00 | 661 | 0.669 | 439 | 1.9 | 15.983 | C |
| 4 | 521 | 423 | 0.00 | 655 | 0.796 | 514 | 3.5 | 24.409 | C |

08:30-08:45

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity <br> (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 540 | 596 | 0.00 | 582 | 0.926 | 520 | 7.3 | 46.800 | E |
| 2 | 563 | 521 | 0.00 | 614 | 0.916 | 544 | 6.9 | 43.028 | E |
| 3 | 542 | 487 | 0.00 | 628 | 0.863 | 529 | 5.0 | 33.016 | D |
| 4 | 639 | 509 | 0.00 | 619 | 1.032 | 590 | 15.5 | 75.075 | F |

08:45-09:00

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 540 | 609 | 0.00 | 577 | 0.935 | 532 | 9.2 | 65.344 | F |
| 2 | 563 | 533 | 0.00 | 609 | 0.924 | 556 | 8.5 | 58.395 | F |
| 3 | 542 | 498 | 0.00 | 623 | 0.869 | 539 | 5.7 | 40.199 | E |
| 4 | 639 | 518 | 0.00 | 615 | 1.038 | 604 | 24.2 | 132.968 | F |

09:00-09:15

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity <br> (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 440 | 579 | 0.00 | 590 | 0.747 | 464 | 3.3 | 32.544 | D |
| 2 | 459 | 480 | 0.00 | 631 | 0.728 | 482 | 2.9 | 26.979 | D |
| 3 | 442 | 432 | 0.00 | 651 | 0.680 | 456 | 2.2 | 19.595 | C |
| 4 | 521 | 442 | 0.00 | 647 | 0.806 | 597 | 5.2 | 83.069 | F |

09:15-09:30

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity <br> (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 369 | 446 | 0.00 | 645 | 0.572 | 376 | 1.4 | 13.733 | B |
| 2 | 385 | 382 | 0.00 | 672 | 0.573 | 391 | 1.4 | 13.062 | B |
| 3 | 370 | 351 | 0.00 | 685 | 0.541 | 375 | 1.2 | 11.741 | B |
| 4 | 437 | 361 | 0.00 | 681 | 0.642 | 450 | 1.9 | 16.445 | C |

## 2018, PM

## Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Vehicle Mix | HV\% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in <br> PCUs or Vehs. |  |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | 42.18 | E |

## Junction Network Options

| Driving side | Lighting |
| :---: | :---: |
| Left | Normal/unknown |

## Traffic Demand

## Demand Set Details

| ID | Scenario name | Time Period name | Traffic profile type | Start time (HH:mm) | Finish time (HH:mm) | Time segment length (min) |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: |
| D2 | 2018 | PM | ONE HOUR | $17: 00$ | $18: 30$ | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

## Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Average Demand (PCU/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | $\checkmark$ | 505 | 100.000 |
| $\mathbf{2}$ |  | $\checkmark$ | 366 | 100.000 |
| $\mathbf{3}$ |  | $\checkmark$ | 466 | 100.000 |
| $\mathbf{4}$ |  | $\checkmark$ | 495 | 100.000 |

Demand overview (Pedestrians)

| Arm | Average pedestrian flow (Ped/hr) |
| :---: | :---: |
| $\mathbf{1}$ | 50.00 |
| $\mathbf{2}$ | 50.00 |
| $\mathbf{3}$ | 50.00 |
| $\mathbf{4}$ | 50.00 |

## Origin-Destination Data

Demand (PCU/hr)

|  | To |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ |
|  | $\mathbf{1}$ | 0 | 75 | 228 | 202 |
|  | $\mathbf{2}$ | 99 | 0 | 77 | 190 |
|  | $\mathbf{3}$ | 238 | 79 | 0 | 149 |
|  | $\mathbf{4}$ | 208 | 167 | 120 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ |
|  | $\mathbf{1}$ | 0 | 0 | 0 | 0 |
|  | $\mathbf{2}$ | 0 | 0 | 0 | 0 |
|  | $\mathbf{3}$ | 0 | 0 | 0 | 0 |
|  | $\mathbf{4}$ | $\mathbf{0}$ | 0 | 0 | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (PCU) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.85 | 34.99 | 5.1 | D |
| $\mathbf{2}$ | 0.72 | 22.76 | 2.5 | C |
| $\mathbf{3}$ | 0.90 | 53.99 | 7.2 | F |
| $\mathbf{4}$ | 0.91 | 52.75 | 7.4 | F |

## Main Results for each time segment

17:00-17:15

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity <br> (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 380 | 272 | 37.64 | 713 | 0.533 | 376 | 1.1 | 10.548 | B |
| 2 | 276 | 409 | 37.64 | 652 | 0.423 | 273 | 0.7 | 9.428 | A |
| 3 | 351 | 366 | 37.64 | 663 | 0.530 | 346 | 1.1 | 11.240 | B |
| 4 | 373 | 309 | 37.64 | 685 | 0.544 | 368 | 1.2 | 11.177 | B |

17:15-17:30

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 454 | 326 | 44.95 | 688 | 0.660 | 451 | 1.9 | 15.026 | C |
| 2 | 329 | 491 | 44.95 | 613 | 0.537 | 327 | 1.1 | 12.525 | B |
| 3 | 419 | 439 | 44.95 | 624 | 0.672 | 416 | 1.9 | 17.018 | C |
| 4 | 445 | 371 | 44.95 | 650 | 0.684 | 441 | 2.0 | 16.929 | C |

## 17:30-17:45

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 556 | 391 | 55.05 | 657 | 0.847 | 545 | 4.6 | 29.690 | D |
| 2 | 403 | 592 | 55.05 | 564 | 0.714 | 398 | 2.3 | 21.081 | C |
| 3 | 513 | 533 | 55.05 | 572 | 0.897 | 497 | 6.0 | 41.118 | E |
| 4 | 545 | 446 | 55.05 | 606 | 0.899 | 528 | 6.2 | 39.889 | E |

17:45-18:00

| Arm | Total Demand <br> $\mathbf{( P C U / h r})$ | Circulating flow <br> $(\mathbf{P C U} / \mathbf{h r})$ | Pedestrian <br> demand <br> $(\mathbf{P e d} / \mathbf{h r})$ | Capacity <br> $(\mathbf{P C U} / \mathbf{h r})$ | $\mathbf{R F C}$ | Throughput <br> $(\mathbf{P C U} / \mathbf{h r})$ | End queue <br> $(\mathbf{P C U})$ | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 556 | 399 | 55.05 | 653 | 0.852 | 554 | 5.1 | 34.989 | D |
| $\mathbf{2}$ | 403 | 603 | 55.05 | 559 | 0.720 | 402 | 2.5 | 22.762 | C |
| $\mathbf{3}$ | 513 | 539 | 55.05 | 568 | 0.904 | 508 | 7.2 | 53.994 | F |
| $\mathbf{4}$ | 545 | 455 | 55.05 | 600 | 0.908 | 540 | 7.4 | 52.752 | F |

18:00-18:15

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity <br> (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 454 | 344 | 44.95 | 679 | 0.668 | 466 | 2.1 | 17.690 | C |
| 2 | 329 | 509 | 44.95 | 605 | 0.544 | 334 | 1.2 | 13.532 | B |
| 3 | 419 | 450 | 44.95 | 617 | 0.679 | 439 | 2.2 | 22.059 | C |
| 4 | 445 | 389 | 44.95 | 640 | 0.695 | 465 | 2.4 | 22.513 | C |

18:15-18:30

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Pedestrian demand (Ped/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 380 | 279 | 37.64 | 710 | 0.536 | 384 | 1.2 | 11.177 | B |
| 2 | 276 | 418 | 37.64 | 647 | 0.426 | 277 | 0.8 | 9.780 | A |
| 3 | 351 | 373 | 37.64 | 659 | 0.532 | 355 | 1.2 | 12.013 | B |
| 4 | 373 | 317 | 37.64 | 681 | 0.547 | 377 | 1.2 | 12.027 | B |

Full Input Data And Results
Full Input Data And Results

## User and Project Details

| Project: | R132 Roundabout Reconfiguration to Signalised Junction <br> Hitle: <br> Location: |
| :--- | :--- |
| Client: | Function of Hamilton Road, Harry Reynolds Road, Dublin Street |

## Network Layout Diagram




Phase Input Data

| Phase Name | Phase Type | Assoc. Phase | Street Min | Cont Min |
| :---: | :---: | :---: | :---: | :---: |
| A | Traffic |  | 7 | 7 |
| B | Traffic |  | 7 | 7 |
| C | Traffic |  | 7 | 7 |
| D | Traffic |  | 7 | 7 |
| E | Pedestrian |  | 7 | 7 |
| F | Pedestrian |  | 7 | 7 |
| G | Pedestrian |  | 7 | 7 |

Phase Intergreens Matrix


## Phases in Stage

| Stage No. | Phases in Stage |
| :---: | :--- |
| 1 | B D |
| 2 | A C |
| 3 | E F G |

## Stage Diagram



## Phase Delays

| Term. Stage | Start Stage | Phase | Type | Value | Cont value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| There are no Phase Delays defined |  |  |  |  |  |

Prohibited Stage Change

|  | To Stage |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  | 1 | 2 | 3 |
| From | 1 |  | 6 | 8 |
| Stage | 2 | 8 |  | 8 |
|  |  | 3 | 8 | 8 |

Full Input Data And Results

## Give-Way Lane Input Data

| Junction: Unnamed Junction |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Movement | Max Flow <br> when <br> Giving Way <br> (PCU/Hr) | Min Flow <br> when <br> Giving Way <br> (PCU/Hr) | Opposing <br> Lane | Opp. Lane <br> Coeff. | Opp. <br> Mvmnts. | Right Turn <br> Storage (PCU) | Non-Blocking <br> Storage <br> (PCU) | RTF | Right Turn <br> Move up (s) | Max Turns <br> in Intergreen <br> (PCU) |
| $1 / 3$ <br> (Hamilton Road) | $8 / 1$ (Right) | 1439 | 0 | $3 / 2$ | 1.09 | All | 2.00 | - | 0.50 | 2 | 2.00 |
| $2 / 2$ <br> (R132) | $5 / 1$ (Right) | 1439 | 0 | $4 / 1$ | 1.09 | All | 2.00 | - | 0.50 | 2 | 2.00 |
| $3 / 3$ <br> $(L 1390)$ | $6 / 1$ (Right) | 1439 | 0 | $1 / 2$ | 1.09 | All | 2.00 | - | 0.50 | 2 | 2.00 |
| $4 / 2$ <br> (Dublin Street) | $7 / 1$ (Right) | 1439 | 0 | $2 / 1$ | 1.09 | All | 2.00 | - | 0.50 | 2 | 2.00 |

Full Input Data And Results
Lane Input Data

| Junction: Unnamed Junction |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Type | Phases | Start Disp. | End Disp. | Physical Length (PCU) | Sat Flow Type | Def User Saturation Flow (PCU/Hr) | Lane <br> Width (m) | Gradient | Nearside Lane | Turns | Turning Radius (m) |
| 1/1 (Hamilton Road) | U | A | 2 | 3 | 60.0 | Geom | - | 3.25 | 0.00 | Y | Arm 6 Left | 10.00 |
| $\begin{gathered} 1 / 2 \\ \text { (Hamilton } \\ \text { Road) } \end{gathered}$ | U | A | 2 | 3 | 60.0 | Geom | - | 3.25 | 0.00 | Y | Arm 7 <br> Ahead | 10.00 |
| $\begin{gathered} 1 / 3 \\ \text { (Hamilton } \end{gathered}$ Road) | 0 | A | 2 | 3 | 60.0 | Geom | - | 3.25 | 0.00 | N | Arm 8 Right | 15.00 |
| $\begin{gathered} 2 / 1 \\ \text { (R132) } \end{gathered}$ | U | B | 2 | 3 | 60.0 | Geom | - | 3.50 | 0.00 | Y | Arm 7 <br> Left <br> Arm 8 <br> Ahead | Inf <br> Inf |
| $\begin{gathered} 2 / 2 \\ (\mathrm{R} 132) \end{gathered}$ | 0 | B | 2 | 3 | 13.9 | Geom | - | 3.50 | 0.00 | Y | Arm 5 Right | 21.00 |
| $\begin{gathered} 3 / 1 \\ \text { (L1390) } \end{gathered}$ | U | C | 2 | 3 | 60.0 | Geom | - | 3.25 | 0.00 | Y | Arm 8 Left | 10.00 |
| $\begin{gathered} 3 / 2 \\ (L 1390) \end{gathered}$ | U | C | 2 | 3 | 60.0 | Geom | - | 3.25 | 0.00 | Y | Arm 5 Ahead | Inf |
| $\begin{gathered} 3 / 3 \\ (\mathrm{~L} 1390) \end{gathered}$ | 0 | C |  | 3 | 60.0 | Geom | - | 3.50 | 0.00 | Y | Arm 6 <br> Right | 16.50 |
| 4/1 |  |  |  |  |  |  |  |  |  |  | Arm 5 Left | 9.50 |
|  |  |  |  |  |  |  |  |  |  |  | Arm 6 Ahead | Inf |
| $\begin{gathered} 4 / 2 \\ \text { (Dublin } \\ \text { Street) } \end{gathered}$ | 0 | D | 2 | 3 | 5.0 | Geom | - | 3.50 | 0.00 | Y | Arm 7 <br> Right | 27.00 |
| 5/1 | U |  | 2 | 3 | 60.0 | Geom | - | 3.50 | 0.00 | Y |  |  |
| 6/1 | U |  | 2 | 3 | 60.0 | Geom | - | 3.50 | 0.00 | Y |  |  |
| 7/1 | U |  | 2 | 3 | 60.0 | Inf | - | - | - | - | - | - |
| 8/1 | U |  | 2 | 3 | 60.0 | Inf | - | - | - | - | - | - |

## Traffic Flow Groups

| Flow Group | Start Time | End Time | Duration | Formula |
| :---: | :---: | :---: | :---: | :---: |
| 1: 'AM Peak' | $08: 00$ | $09: 00$ | $01: 00$ |  |
| 2: 'PM Peak' | $17: 00$ | $18: 00$ | $01: 00$ |  |
| 3: 'AM - 15\% growth' | $08: 00$ | $09: 00$ | $01: 00$ | F1*1.15 |
| 4: 'AM - 20\% growth' | $08: 00$ | $09: 00$ | $01: 00$ | F3*1.2 |

Full Input Data And Results

Scenario 1: 'AM Peak' (FG1: 'AM Peak', Plan 1: 'Network Control Plan 1') Traffic Flows, Desired Desired Flow :

|  | Destination |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | A | B | C | D | Tot. |  |
|  | A | 0 | 115 | 245 | 130 | 490 |  |
|  | B | 139 | 0 | 183 | 189 | 511 |  |
|  | C | 184 | 151 | 0 | 157 | 492 |  |
|  | D | 154 | 305 | 121 | 0 | 580 |  |
|  | Tot. | 477 | 571 | 549 | 476 | 2073 |  |

## Traffic Lane Flows

| Lane | Scenario 1: <br> AM Peak |
| :---: | :---: |
| Junction: Unnamed Junction |  |
| 1/1 | 183 |
| 1/2 | 189 |
| 1/3 | 139 |
| $\begin{gathered} 2 / 1 \\ \text { (with short) } \end{gathered}$ | $\begin{gathered} \text { 492(In) } \\ 341 \text { (Out) } \end{gathered}$ |
| $\begin{gathered} 2 / 2 \\ \text { (short) } \end{gathered}$ | 151 |
| 3/1 | 154 |
| 3/2 | 305 |
| 3/3 | 121 |
| $\begin{gathered} 4 / 1 \\ \text { (with short) } \end{gathered}$ | $\begin{gathered} \text { 490(In) } \\ 360 \text { (Out) } \end{gathered}$ |
| $\begin{gathered} 4 / 2 \\ \text { (short) } \end{gathered}$ | 130 |
| 5/1 | 571 |
| 6/1 | 549 |
| 7/1 | 476 |
| 8/1 | 477 |

Full Input Data And Results
Lane Saturation Flows

| Junction: Unnamed Junction |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $\begin{gathered} 1 / 1 \\ \text { (Hamilton Road) } \end{gathered}$ | 3.25 | 0.00 | Y | Arm 6 Left | 10.00 | 100.0 \% | 1687 | 1687 |
| $\begin{gathered} 1 / 2 \\ \text { (Hamilton Road) } \end{gathered}$ | 3.25 | 0.00 | Y | Arm 7 Ahead | 10.00 | 100.0 \% | 1687 | 1687 |
| $\begin{gathered} 1 / 3 \\ \text { (Hamilton Road) } \end{gathered}$ | 3.25 | 0.00 | N | Arm 8 Right | 15.00 | 100.0 \% | 1891 | 1891 |
| $\begin{gathered} 2 / 1 \\ \text { (R132) } \end{gathered}$ | 3.50 | 0.00 | Y | Arm 7 Left Arm 8 Ahead | Inf <br> Inf | $\begin{aligned} & 46.0 \% \\ & 54.0 \% \end{aligned}$ | 1965 | 1965 |
| $\begin{gathered} 2 / 2 \\ (R 132) \end{gathered}$ | 3.50 | 0.00 | Y | Arm 5 Right |  | 100.0 \% | 1834 | 1834 |
| $\begin{gathered} 3 / 1 \\ (\mathrm{~L} 1390) \end{gathered}$ | 3.25 | 0.00 | Y | Arm 8 Left | 10.00 | 100.0 \% | 1687 | 1687 |
| $\begin{gathered} 3 / 2 \\ (L 1390) \end{gathered}$ | 3.25 | 0.00 | Y | Arm 5 Ahead | Inf | 100.0 \% | 1940 | 1940 |
| $\begin{gathered} 3 / 3 \\ (\mathrm{~L} 1390) \end{gathered}$ | 3.50 | 0.00 | Y | Arm 6 Right | 16.50 | 100.0 \% | 1801 | 1801 |
| 4/1 |  |  |  | Arm 5 Left | 9.50 | 31.9 \% |  |  |
| (Dublin Street) | 3.25 | 0.00 | $Y$ | Arm 6 Ahead | Inf | 68.1 \% | 1847 | 1847 |
| $\begin{gathered} 4 / 2 \\ \text { (Dublin Street) } \end{gathered}$ | 3.50 | 0.00 | Y | Arm 7 Right | 27.00 | 100.0 \% | 1862 | 1862 |
| 5/1 | 3.50 | 0.00 | Y |  |  |  | 1965 | 1965 |
| 6/1 | 3.50 | 0.00 | Y |  |  |  | 1965 | 1965 |
| 7/1 |  |  | Infinite S | aturation Flow |  |  | Inf | Inf |
| 8/1 |  |  | Infinite S | aturation Flow |  |  | Inf | Inf |

Scenario 2: 'AM Peak with 15\% growth' (FG3: 'AM - 15\% growth', Plan 1: 'Network Control Plan 1') Traffic Flows, Desired
Desired Flow :

| Origin | Destination |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | A | B | C | D | Tot. |  |
|  | A | 0 | 132 | 282 | 150 | 564 |  |
|  | B | 160 | 0 | 210 | 217 | 587 |  |
|  | C | 212 | 174 | 0 | 181 | 567 |  |
|  | D | 177 | 351 | 139 | 0 | 667 |  |
|  | Tot. | 549 | 657 | 631 | 548 | 2385 |  |

Full Input Data And Results
Traffic Lane Flows

| Lane | Scenario 2: AM Peak with 15\% growth |
| :---: | :---: |
| Junction: Unnamed Junction |  |
| 1/1 | 210 |
| 1/2 | 217 |
| 1/3 | 160 |
| $\begin{gathered} 2 / 1 \\ \text { (with short) } \end{gathered}$ | $\begin{gathered} 567 \text { (In) } \\ \text { 393(Out) } \end{gathered}$ |
| $\begin{gathered} 2 / 2 \\ \text { (short) } \end{gathered}$ | 174 |
| 3/1 | 177 |
| 3/2 | 351 |
| 3/3 | 139 |
| 4/1 <br> (with short) | $\begin{gathered} 564(\text { In }) \\ 414(\text { Out }) \end{gathered}$ |
| $\begin{gathered} 4 / 2 \\ \text { (short) } \end{gathered}$ | 150 |
| 5/1 | 657 |
| 6/1 | 631 |
| 7/1 | 548 |
| 8/1 | 549 |

Full Input Data And Results
Lane Saturation Flows

| Junction: Unnamed Junction |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $\begin{gathered} 1 / 1 \\ \text { (Hamilton Road) } \end{gathered}$ | 3.25 | 0.00 | Y | Arm 6 Left | 10.00 | 100.0 \% | 1687 | 1687 |
| $\begin{gathered} 1 / 2 \\ \text { (Hamilton Road) } \end{gathered}$ | 3.25 | 0.00 | Y | Arm 7 Ahead | 10.00 | 100.0 \% | 1687 | 1687 |
| $\begin{gathered} 1 / 3 \\ \text { (Hamilton Road) } \end{gathered}$ | 3.25 | 0.00 | N | Arm 8 Right | 15.00 | 100.0 \% | 1891 | 1891 |
| $\begin{gathered} 2 / 1 \\ \text { (R132) } \end{gathered}$ | 3.50 | 0.00 | Y | Arm 7 Left Arm 8 Ahead | Inf Inf | $\begin{aligned} & 46.1 \% \\ & 53.9 \text { \% } \end{aligned}$ | 1965 | 1965 |
| $\begin{gathered} 2 / 2 \\ (R 132) \end{gathered}$ | 3.50 | 0.00 | Y | Arm 5 Right |  | 100.0 \% | 1834 | 1834 |
| $\begin{gathered} 3 / 1 \\ (\mathrm{~L} 1390) \end{gathered}$ | 3.25 | 0.00 | Y | Arm 8 Left | 10.00 | 100.0 \% | 1687 | 1687 |
| $\begin{gathered} 3 / 2 \\ (L 1390) \end{gathered}$ | 3.25 | 0.00 | Y | Arm 5 Ahead | Inf | 100.0 \% | 1940 | 1940 |
| $\begin{gathered} 3 / 3 \\ (\mathrm{~L} 1390) \end{gathered}$ | 3.50 | 0.00 | Y | Arm 6 Right | 16.50 | 100.0 \% | 1801 | 1801 |
| 4/1 |  |  |  | Arm 5 Left | 9.50 | 31.9 \% |  |  |
| (Dublin Street) | 3.25 | 0.00 | $Y$ | Arm 6 Ahead | Inf | 68.1 \% | 1847 | 1847 |
| $\begin{gathered} 4 / 2 \\ \text { (Dublin Street) } \end{gathered}$ | 3.50 | 0.00 | Y | Arm 7 Right | 27.00 | 100.0 \% | 1862 | 1862 |
| 5/1 | 3.50 | 0.00 | Y |  |  |  | 1965 | 1965 |
| 6/1 | 3.50 | 0.00 | Y |  |  |  | 1965 | 1965 |
| 7/1 |  |  | Infinite S | aturation Flow |  |  | Inf | Inf |
| 8/1 |  |  | Infinite S | aturation Flow |  |  | Inf | Inf |

Scenario 3: 'AM Peak with 20\% growth' (FG4: 'AM - 20\% growth', Plan 1: 'Network Control Plan 1') Traffic Flows, Desired
Desired Flow :

| Origin | Destination |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | A | B | C | D | Tot. |  |
|  | A | 0 | 158 | 338 | 180 | 676 |  |
|  | B | 192 | 0 | 252 | 260 | 704 |  |
|  | C | 254 | 209 | 0 | 217 | 680 |  |
|  | D | 212 | 421 | 167 | 0 | 800 |  |
|  | Tot. | 658 | 788 | 757 | 657 | 2860 |  |

Full Input Data And Results
Traffic Lane Flows

| Lane | Scenario 3: AM Peak with 20\% growth |
| :---: | :---: |
| Junction: Unnamed Junction |  |
| 1/1 | 252 |
| 1/2 | 260 |
| 1/3 | 192 |
| $\begin{gathered} 2 / 1 \\ \text { (with short) } \end{gathered}$ | $\begin{gathered} 680 \text { (In) } \\ 471 \text { (Out) } \end{gathered}$ |
| $\begin{gathered} 2 / 2 \\ \text { (short) } \end{gathered}$ | 209 |
| 3/1 | 212 |
| 3/2 | 421 |
| 3/3 | 167 |
| 4/1 <br> (with short) | $\begin{gathered} \text { 676(In) } \\ 496(\text { Out) } \end{gathered}$ |
| $\begin{gathered} 4 / 2 \\ \text { (short) } \end{gathered}$ | 180 |
| 5/1 | 788 |
| 6/1 | 757 |
| 7/1 | 657 |
| 8/1 | 658 |

Full Input Data And Results
Lane Saturation Flows

| Junction: Unnamed Junction |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $\begin{gathered} 1 / 1 \\ \text { (Hamilton Road) } \end{gathered}$ | 3.25 | 0.00 | Y | Arm 6 Left | 10.00 | 100.0 \% | 1687 | 1687 |
| $\begin{gathered} 1 / 2 \\ \text { (Hamilton Road) } \end{gathered}$ | 3.25 | 0.00 | Y | Arm 7 Ahead | 10.00 | 100.0 \% | 1687 | 1687 |
| $\begin{gathered} 1 / 3 \\ \text { (Hamilton Road) } \end{gathered}$ | 3.25 | 0.00 | N | Arm 8 Right | 15.00 | 100.0 \% | 1891 | 1891 |
| $\begin{gathered} 2 / 1 \\ \text { (R132) } \end{gathered}$ | 3.50 | 0.00 | Y | Arm 7 Left Arm 8 Ahead | Inf Inf | 46.1 \% <br> 53.9 \% | 1965 | 1965 |
| $\begin{gathered} 2 / 2 \\ (\mathrm{R} 132) \end{gathered}$ | 3.50 | 0.00 | Y | Arm 5 Right |  |  | 1834 | 1834 |
| $\begin{gathered} 3 / 1 \\ \text { (L1390) } \end{gathered}$ | 3.25 | 0.00 | Y | Arm 8 Left | 10.00 | 100.0 \% | 1687 | 1687 |
| $\begin{gathered} 3 / 2 \\ (\mathrm{~L} 1390) \end{gathered}$ | 3.25 | 0.00 | Y | Arm 5 Ahead | Inf | 100.0 \% | 1940 | 1940 |
| $\begin{gathered} 3 / 3 \\ (L 1390) \end{gathered}$ | 3.50 | 0.00 | Y | Arm 6 Right | 16.50 | 100.0 \% | 1801 | 1801 |
|  |  |  |  | Arm 5 Left | 9.50 | 31.9 \% |  |  |
| (Dublin Street) | 3.25 | 0.00 | Y | Arm 6 Ahead | Inf | 68.1 \% | 1847 | 1847 |
| $\begin{gathered} 4 / 2 \\ \text { (Dublin Street) } \end{gathered}$ | 3.50 | 0.00 | Y | Arm 7 Right | 27.00 | 100.0 \% | 1862 | 1862 |
| 5/1 | 3.50 | 0.00 | Y |  |  |  | 1965 | 1965 |
| 6/1 | 3.50 | 0.00 | Y |  |  |  | 1965 | 1965 |
| 7/1 |  |  | Infinite S | aturation Flow |  |  | Inf | Inf |
| 8/1 |  |  | Infinite S | aturation Flow |  |  | Inf | Inf |

## Scenario 1: 'AM Peak' (FG1: 'AM Peak', Plan 1: 'Network Control Plan 1') <br> Stage Sequence Diagram



Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
| :---: | :---: | :---: | :---: |
| Duration | 30 | 31 | 7 |
| Change Point | 0 | 38 | 75 |

Signal Timings Diagram


Full Input Data And Results

## Network Layout Diagram



Network Results

| Item | Lane Description | Lane <br> Type | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand <br> Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Harry Reynolds Road Pedestrian and Cycle Scheme | - | - | N/A | - | - |  | - | - | - | - | - | - | 69.6\% |
| Unnamed Junction | - | - | N/A | - | - |  | - | - | - | - | - | - | 69.6\% |
| 1/1 | Hamilton Road Left | U | N/A | N/A | A |  | 1 | 31 | - | 183 | 1687 | 600 | 30.5\% |
| 1/2 | Hamilton Road Ahead | U | N/A | N/A | A |  | 1 | 31 | - | 189 | 1687 | 600 | 31.5\% |
| 1/3 | Hamilton Road Right | 0 | N/A | N/A | A |  | 1 | 31 | - | 139 | 1891 | 314 | 44.3\% |
| $2 / 1+2 / 2$ | R132 Right Left Ahead | U+O | N/A | N/A | B |  | 1 | 30 | - | 492 | 1965:1834 | 647+221 | $\begin{gathered} 52.7: \\ 68.3 \% \end{gathered}$ |
| 3/1 | L1390 Left | U | N/A | N/A | C |  | 1 | 31 | - | 154 | 1687 | 600 | 25.7\% |
| 3/2 | L1390 Ahead | U | N/A | N/A | C |  | 1 | 31 | - | 305 | 1940 | 690 | 44.2\% |
| 3/3 | L1390 Right | 0 | N/A | N/A | C |  | 1 | 31 | - | 121 | 1801 | 389 | 31.1\% |
| 4/1+4/2 | Dublin Street Left Ahead Right | U+O | N/A | N/A | D |  | 1 | 30 | - | 490 | 1847:1862 | $517+187$ | $\begin{aligned} & 69.6 \text { : } \\ & 69.6 \% \end{aligned}$ |
| 5/1 |  | U | N/A | N/A | - |  | - | - | - | 571 | 1965 | 1965 | 29.1\% |
| 6/1 |  | U | N/A | N/A | - |  | - | - | - | 549 | 1965 | 1965 | 27.9\% |
| 7/1 |  | U | N/A | N/A | - |  | - | - | - | 476 | Inf | Inf | 0.0\% |
| 8/1 |  | U | N/A | N/A | - |  | - | - | - | 477 | Inf | Inf | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + Oversat Delay (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Harry Reynolds Road Pedestrian and Cycle Scheme | - | - | 541 | 0 | 0 | 13.0 | 3.8 | 1.3 | 18.2 | - | - | - | - |
| Unnamed Junction | - | - | 541 | 0 | 0 | 13.0 | 3.8 | 1.3 | 18.2 | - | - | - | - |
| 1/1 | 183 | 183 | - | - | - | 1.1 | 0.2 | - | 1.3 | 25.3 | 3.3 | 0.2 | 3.5 |
| 1/2 | 189 | 189 | - | - | - | 1.1 | 0.2 | - | 1.3 | 25.4 | 3.4 | 0.2 | 3.6 |
| 1/3 | 139 | 139 | 139 | 0 | 0 | 0.9 | 0.4 | 0.3 | 1.6 | 40.5 | 2.9 | 0.4 | 3.3 |
| 2/1+2/2 | 492 | 492 | 151 | 0 | 0 | 3.3 | 0.7 | 0.5 | 4.4 | 32.5 | 6.7 | 0.7 | 7.4 |
| 3/1 | 154 | 154 | - | - | - | 0.9 | 0.2 | - | 1.1 | 24.6 | 2.7 | 0.2 | 2.9 |
| 3/2 | 305 | 305 | - | - | - | 1.9 | 0.4 | - | 2.3 | 26.8 | 5.8 | 0.4 | 6.2 |
| 3/3 | 121 | 121 | 121 | 0 | 0 | 0.7 | 0.2 | 0.2 | 1.1 | 33.1 | 2.3 | 0.2 | 2.5 |
| 4/1+4/2 | 490 | 490 | 130 | 0 | 0 | 3.3 | 1.1 | 0.3 | 4.7 | 34.8 | 8.4 | 1.1 | 9.5 |
| 5/1 | 571 | 571 | - | - | - | 0.0 | 0.2 | - | 0.2 | 1.3 | 0.0 | 0.2 | 0.2 |
| 6/1 | 549 | 549 | - | - | - | 0.0 | 0.2 | - | 0.2 | 1.3 | 0.0 | 0.2 | 0.2 |
| $7 / 1$ | 476 | 476 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 8/1 | 477 | 477 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| C1 |  |  | PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): |  | $\begin{aligned} & 29.3 \\ & 29.3 \end{aligned}$ |   <br> Total Delay for Signalled Lanes (pcuHr): 17.80 <br> Total Delay Over All Lanes(pcuHr): 18.20 |  |  | Cycle Time (s): 90 |  |  |  |  |

Full Input Data And Results
Scenario 2: 'AM Peak with 15\% growth' (FG3: 'AM - 15\% growth', Plan 1: 'Network Control Plan 1')


Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
| :---: | :---: | :---: | :---: |
| Duration | 30 | 31 | 7 |
| Change Point | 0 | 38 | 75 |

Signal Timings Diagram


Full Input Data And Results

## Network Layout Diagram



Network Results

| Item | Lane Description | Lane <br> Type | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand <br> Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Harry Reynolds Road Pedestrian and Cycle Scheme | - | - | N/A | - | - |  | - | - | - | - | - | - | 105.0\% |
| Unnamed Junction | - | - | N/A | - | - |  | - | - | - | - | - | - | 105.0\% |
| 1/1 | Hamilton Road Left | U | N/A | N/A | A |  | 1 | 31 | - | 210 | 1687 | 600 | 35.0\% |
| 1/2 | Hamilton Road Ahead | U | N/A | N/A | A |  | 1 | 31 | - | 217 | 1687 | 600 | 36.2\% |
| 1/3 | Hamilton Road Right | 0 | N/A | N/A | A |  | 1 | 31 | - | 160 | 1891 | 280 | 57.2\% |
| $2 / 1+2 / 2$ | R132 Right <br> Left Ahead | U+O | N/A | N/A | B |  | 1 | 30 | - | 567 | 1965:1834 | 647+166 | $\begin{gathered} 60.7 \text { : } \\ 105.0 \% \end{gathered}$ |
| 3/1 | L1390 Left | U | N/A | N/A | C |  | 1 | 31 | - | 177 | 1687 | 600 | 29.5\% |
| 3/2 | L1390 Ahead | U | N/A | N/A | C |  | 1 | 31 | - | 351 | 1940 | 690 | 50.9\% |
| 3/3 | L1390 Right | 0 | N/A | N/A | C |  | 1 | 31 | - | 139 | 1801 | 364 | 38.2\% |
| 4/1+4/2 | Dublin Street Left Ahead Right | U+O | N/A | N/A | D |  | 1 | 30 | - | 564 | 1847:1862 | $517+187$ | $\begin{aligned} & 80.1 \text { : } \\ & 80.1 \% \end{aligned}$ |
| 5/1 |  | U | N/A | N/A | - |  | - | - | - | 657 | 1965 | 1965 | 33.0\% |
| 6/1 |  | U | N/A | N/A | - |  | - | - | - | 631 | 1965 | 1965 | 32.1\% |
| 7/1 |  | U | N/A | N/A | - |  | - | - | - | 548 | Inf | Inf | 0.0\% |
| 8/1 |  | U | N/A | N/A | - |  | - | - | - | 549 | Inf | Inf | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + Oversat Delay (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Harry Reynolds Road Pedestrian and Cycle Scheme | - | - | 557 | 0 | 58 | 16.1 | 10.0 | 1.7 | 27.8 | - | - | - | - |
| Unnamed Junction | - | - | 557 | 0 | 58 | 16.1 | 10.0 | 1.7 | 27.8 | - | - | - | - |
| 1/1 | 210 | 210 | - | - | - | 1.2 | 0.3 | - | 1.5 | 26.0 | 3.8 | 0.3 | 4.1 |
| 1/2 | 217 | 217 | - | - | - | 1.3 | 0.3 | - | 1.6 | 26.2 | 4.0 | 0.3 | 4.3 |
| 1/3 | 160 | 160 | 160 | 0 | 0 | 1.1 | 0.7 | 0.4 | 2.1 | 47.9 | 3.4 | 0.7 | 4.1 |
| 2/1+2/2 | 567 | 559 | 108 | 0 | 58 | 4.4 | 5.3 | 0.6 | 10.3 | 65.5 | 8.0 | 5.3 | 13.3 |
| 3/1 | 177 | 177 | - | - | - | 1.0 | 0.2 | - | 1.2 | 25.1 | 3.1 | 0.2 | 3.4 |
| 3/2 | 351 | 351 | - | - | - | 2.2 | 0.5 | - | 2.7 | 28.1 | 6.8 | 0.5 | 7.3 |
| 3/3 | 139 | 139 | 139 | 0 | 0 | 0.8 | 0.3 | 0.2 | 1.4 | 36.0 | 2.7 | 0.3 | 3.0 |
| 4/1+4/2 | 564 | 564 | 150 | 0 | 0 | 4.0 | 2.0 | 0.4 | 6.4 | 40.7 | 10.7 | 2.0 | 12.7 |
| 5/1 | 649 | 649 | - | - | - | 0.0 | 0.2 | - | 0.2 | 1.4 | 0.0 | 0.2 | 0.2 |
| 6/1 | 631 | 631 | - | - | - | 0.0 | 0.2 | - | 0.2 | 1.3 | 0.0 | 0.2 | 0.2 |
| 7/1 | 548 | 548 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 8/1 | 549 | 549 |  | (1) | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| C1 |  |  | PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): |  | $\begin{aligned} & -16.7 \\ & -16.7 \end{aligned}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  |  | Cycle Time (s): 90 |  |  |  |  |

Full Input Data And Results
Scenario 3: 'AM Peak with 20\% growth' (FG4: 'AM - 20\% growth', Plan 1: 'Network Control Plan 1')
Stage Sequence Diagram


Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
| :---: | :---: | :---: | :---: |
| Duration | 21 | 10 | 7 |
| Change Point | 0 | 29 | 45 |

Signal Timings Diagram


Full Input Data And Results

## Network Layout Diagram



Network Results

| Item | Lane Description | Lane <br> Type | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand <br> Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Harry Reynolds Road Pedestrian and Cycle Scheme | - | - | N/A | - | - |  | - | - | - | - | - | - | 160.0\% |
| Unnamed Junction | - | - | N/A | - | - |  | - | - | - | - | - | - | 160.0\% |
| 1/1 | Hamilton Road Left | U | N/A | N/A | A |  | 1 | 10 | - | 252 | 1687 | 309 | 81.5\% |
| 1/2 | Hamilton Road Ahead | U | N/A | N/A | A |  | 1 | 10 | - | 260 | 1687 | 309 | 84.1\% |
| 1/3 | Hamilton Road Right | 0 | N/A | N/A | A |  | 1 | 10 | - | 192 | 1891 | 120 | 160.0\% |
| $2 / 1+2 / 2$ | R132 Right <br> Left Ahead | U+O | N/A | N/A | B |  | 1 | 21 | - | 680 | 1965:1834 | 721+168 | $\begin{gathered} 65.4: \\ 124.2 \% \end{gathered}$ |
| 3/1 | L1390 Left | U | N/A | N/A | C |  | 1 | 10 | - | 212 | 1687 | 309 | 68.5\% |
| 3/2 | L1390 Ahead | U | N/A | N/A | C |  | 1 | 10 | - | 421 | 1940 | 356 | 118.4\% |
| 3/3 | L1390 Right | 0 | N/A | N/A | C |  | 1 | 10 | - | 167 | 1801 | 120 | 139.2\% |
| 4/1+4/2 | Dublin Street Left Ahead Right | U+O | N/A | N/A | D |  | 1 | 21 | - | 676 | 1847:1862 | 572+207 | $\begin{aligned} & 86.8: \\ & 86.8 \% \end{aligned}$ |
| 5/1 |  | U | N/A | N/A | - |  | - | - | - | 788 | 1965 | 1965 | 34.7\% |
| 6/1 |  | U | N/A | N/A | - |  | - | - | - | 757 | 1965 | 1965 | 36.1\% |
| 7/1 |  | U | N/A | N/A | - |  | - | - | - | 657 | Inf | Inf | 0.0\% |
| 8/1 |  | U | N/A | N/A | - |  | - | - | - | 658 | Inf | Inf | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + Oversat Delay (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Harry Reynolds Road Pedestrian and Cycle Scheme | - | - | 271 | 0 | 317 | 21.8 | 129.2 | 1.9 | 152.8 | - | - | - | - |
| Unnamed Junction | - | - | 271 | 0 | 317 | 21.8 | 129.2 | 1.9 | 152.8 | - | - | - | - |
| 1/1 | 252 | 252 | - | - | - | 1.6 | 2.1 | - | 3.7 | 52.9 | 4.0 | 2.1 | 6.0 |
| 1/2 | 260 | 260 | - | - | - | 1.7 | 2.4 | - | 4.1 | 56.9 | 4.1 | 2.4 | 6.5 |
| 1/3 | 192 | 120 | 0 | 0 | 120 | 3.3 | 37.3 | 0.4 | 41.0 | 768.0 | 5.0 | 37.3 | 42.2 |
| 2/1+2/2 | 680 | 639 | 78 | 0 | 90 | 4.1 | 21.9 | 0.7 | 26.7 | 141.5 | 6.4 | 21.9 | 28.3 |
| 3/1 | 212 | 212 | - | - | - | 1.3 | 1.1 | - | 2.4 | 41.0 | 3.3 | 1.1 | 4.4 |
| 3/2 | 421 | 356 | - | - | - | 4.3 | 35.6 | - | 39.9 | 341.2 | 8.1 | 35.6 | 43.7 |
| 3/3 | 167 | 120 | 39 | 0 | 81 | 2.3 | 25.2 | 0.3 | 27.7 | 598.1 | 3.8 | 25.2 | 29.0 |
| 4/1+4/2 | 676 | 676 | 154 | 0 | 26 | 3.1 | 3.1 | 0.5 | 6.7 | 35.6 | 8.3 | 3.1 | 11.4 |
| 5/1 | 682 | 682 | - | - | - | 0.0 | 0.3 | - | 0.3 | 1.4 | 0.0 | 0.3 | 0.3 |
| 6/1 | 710 | 710 | - | - | - | 0.0 | 0.3 | - | 0.3 | 1.4 | 0.0 | 0.3 | 0.3 |
| 7/1 | 657 | 657 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 8/1 | 586 | 586 | - | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| C1 |  |  | PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): |  | $\begin{aligned} & -77.8 \\ & -77.8 \end{aligned}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  |  | Cycle Time (s): 60 |  |  |  |  |

## Junctions 9



Filename: Moylaragh Road - Harry Reynolds Road Cycle Friendly Roundabout.j9
Path: U:\5165984\7 Calcs\72Model
Report generation date: 17/10/2018 11:02:28
"2018, AM
"2018, PM
Summary of junction performance

|  | AM |  |  |  |  | PM |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Queue (PCU) | Delay (s) | RFC | LOS | Queue (PCU) | Delay (s) | RFC | LOS |  |  |
|  | $\mathbf{2 0 1 8}$ |  |  |  |  |  |  |  |  |  |
| Arm 1 | 1.8 | 15.78 | 0.65 | C | 0.6 | 9.42 | 0.39 | A |  |  |
| Arm 2 | 1.8 | 19.95 | 0.65 | C | 0.8 | 11.20 | 0.43 | B |  |  |
| Arm 3 | 0.7 | 12.14 | 0.43 | B | 1.8 | 19.62 | 0.65 | C |  |  |

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.
Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle.
File summary
File Description

| Title | (untitled) |
| :--- | :--- |
| Location |  |
| Site number |  |
| Date | $09 / 05 / 2018$ |
| Version |  |
| Status | (new file) |
| Identifier |  |
| Client |  |
| Jobnumber |  |
| Enumerator | ATKINSMCCARTHY $M C$ MOllins |
| Description |  |

## Units

| Distance units | Speed units | Traffic units input | Traffic units results | Flow units | Average delay units | Total delay units | Rate of delay units |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| m | kph | PCU | PCU | perHour | S | -Min | perMin |

Analysis Options

| Calculate Queue Percentiles | Calculate residual capacity | RFC Threshold | Average Delay threshold (s) | Queue threshold (PCU) |
| :--- | :---: | :---: | :---: | :---: |
|  |  | 0.85 | 36.00 | 20.00 |

## Demand Set Summary

| ID | Scenario name | Time Period name | Traffic profile type | Start time (HH:mm) | Finish time (HH:mm) | Time segment length (min) |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: |
| D1 | 2018 | AM | ONE HOUR | $08: 00$ | $09: 30$ | 15 |
| D2 | 2018 | PM | ONE HOUR | $17: 00$ | $18: 30$ | 15 |

## Analysis Set Details

| ID | Network flow scaling factor (\%) |
| :---: | :---: |
| A1 | 100.000 |

## 2018, AM

## Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Vehicle Mix | HV\% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in <br> PCUs or Vehs. |  |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | 16.38 | C |

## Junction Network Options

| Driving side | Lighting |
| :---: | :---: |
| Left | Normal/unknown |

## Arms

## Arms

| Arm | Name | Description |
| :---: | :---: | :--- |
| $\mathbf{1}$ | untitled |  |
| $\mathbf{2}$ | untitled |  |
| $\mathbf{3}$ | untitled |  |

Roundabout Geometry

| Arm | V - Approach road half- <br> width $(\mathbf{m})$ | E Entry width <br> $(\mathbf{m})$ | I' - Effective flare <br> length $(\mathbf{m})$ | R - Entry radius <br> $(\mathbf{m})$ | D - Inscribed circle <br> diameter $(\mathbf{m})$ | PHI - Conflict (entry) <br> angle (deg) | Exit <br> only |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 3.00 | 3.00 | 0.0 | 7.3 | 32.0 |  |  |
| $\mathbf{2}$ | 3.00 | 3.00 | 0.0 | 4.2 | 32.0 |  |  |
| $\mathbf{3}$ | 3.00 | 3.00 | 0.0 | 3.6 | 32.0 |  |  |

## Slope / Intercept / Capacity

Roundabout Slope and Intercept used in model

| Arm | Final slope | Final intercept (PCU/hr) |
| :---: | :---: | :---: |
| $\mathbf{1}$ | 0.377 | 693 |
| $\mathbf{2}$ | 0.337 | 619 |
| $\mathbf{3}$ | 0.298 | 549 |

The slope and intercept shown above include any corrections and adjustments.

## Traffic Demand

## Demand Set Details

| ID | Scenario name | Time Period name | Traffic profile type | Start time (HH:mm) | Finish time (HH:mm) | Time segment length (min) |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: |
| D1 | 2018 | AM | ONE HOUR | $08: 00$ | $09: 30$ | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

## Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Average Demand (PCU/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | $\checkmark$ | 377 | 100.000 |
| $\mathbf{2}$ |  | $\checkmark$ | 306 | 100.000 |
| $\mathbf{3}$ |  | $\checkmark$ | 203 | 100.000 |

## Origin-Destination Data

Demand (PCU/hr)

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 101 | 276 |
|  | $\mathbf{2}$ | 88 | 0 | 218 |
|  | $\mathbf{3}$ | 82 | 121 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 0 | 0 |
|  | $\mathbf{2}$ | 0 | 0 | 0 |
|  | $\mathbf{3}$ | 0 | 0 | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (PCU) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.65 | 15.78 | 1.8 | C |
| $\mathbf{2}$ | 0.65 | 19.95 | 1.8 | C |
| $\mathbf{3}$ | 0.43 | 12.14 | 0.7 | B |

## Main Results for each time segment

08:00-08:15

| Arm | Total Demand <br> (PCU/hr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput <br> (PCU/hr) | End queue (PCU) | Delay (s) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 284 | 90 | 659 | 0.431 | 281 | 0.7 | LOS |
| $\mathbf{2}$ | 230 | 206 | 550 | 0.419 | 228 | 0.451 |  |
| $\mathbf{3}$ | 153 | 65 | 529 | 0.289 | 11.085 | 0.7 |  |

08:15-08:30

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 339 | 108 | 652 | 0.520 | 338 | 1.1 | 11.405 | B |
| 2 | 275 | 247 | 536 | 0.514 | 274 | 1.0 | 13.681 | B |
| 3 | 182 | 79 | 525 | 0.347 | 182 | 0.5 | 10.470 | B |

08:30-08:45

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 415 | 133 | 643 | 0.646 | 412 | 1.7 | 15.425 | C |
| 2 | 337 | 302 | 517 | 0.651 | 334 | 1.8 | 19.326 | C |
| 3 | 224 | 96 | 520 | 0.430 | 223 | 0.7 | 12.064 | B |

08:45-09:00

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 415 | 133 | 643 | 0.646 | 415 | 1.8 | 15.776 | C |
| 2 | 337 | 304 | 517 | 0.652 | 337 | 1.8 | 19.951 | C |
| 3 | 224 | 97 | 520 | 0.430 | 223 | 0.7 | 12.140 | B |

09:00-09:15

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 339 | 109 | 652 | 0.520 | 342 | 1.1 | 11.708 | B |
| 2 | 275 | 250 | 535 | 0.515 | 278 | 1.1 | 14.184 | B |
| 3 | 182 | 80 | 525 | 0.348 | 183 | 0.5 | 10.564 | B |

09:15-09:30

| Arm | Total Demand <br> (PCU/hr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput <br> (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 284 | 91 | 658 | 0.431 | 285 | 0.8 | 9.681 |  |
| $\mathbf{2}$ | 230 | 209 | 549 | 0.420 | 232 | 0.7 | 11.418 |  |
| $\mathbf{3}$ | 153 | 67 | 529 | 0.289 | 153 | 0.4 | 9.599 | A |

## 2018, PM

## Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Vehicle Mix | HV\% is zero for all movements / time segments. Vehicle Mix matrix should be completed whether working in <br> PCUs or Vehs. |  |

## Junction Network

Junctions

| Junction | Name | Junction Type | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | 14.13 | B |

## Junction Network Options

| Driving side | Lighting |
| :---: | :---: |
| Left | Normal/unknown |

## Traffic Demand

## Demand Set Details

| ID | Scenario name | Time Period name | Traffic profile type | Start time (HH:mm) | Finish time (HH:mm) | Time segment length (min) |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: |
| D2 | 2018 | PM | ONE HOUR | $17: 00$ | $18: 30$ | 15 |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

## Demand overview (Traffic)

| Arm | Linked arm | Use O-D data | Average Demand (PCU/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | $\checkmark$ | 224 | 100.000 |
| $\mathbf{2}$ |  | $\checkmark$ | 224 | 100.000 |
| $\mathbf{3}$ |  | $\checkmark$ | 311 | 100.000 |

## Origin-Destination Data

Demand (PCU/hr)

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 87 | 137 |
|  | $\mathbf{2}$ | 72 | 0 | 152 |
|  | $\mathbf{3}$ | 156 | 155 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | 1 | 2 | 3 |
|  | 1 | 0 | 0 | 0 |
|  | 2 | 0 | 0 | 0 |
|  | 3 | 0 | 0 | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (PCU) | Max LOS |
| :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.39 | 9.42 | 0.6 | A |
| $\mathbf{2}$ | 0.43 | 11.20 | 0.8 | B |
| $\mathbf{3}$ | 0.65 | 19.62 | 1.8 | C |

## Main Results for each time segment

17:00-17:15

| Arm | Total Demand <br> (PCU/hr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput <br> (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 169 | 115 | 649 | 0.260 | 167 | 0.3 |  |  |
| $\mathbf{2}$ | 169 | 102 | 584 | 0.289 | 167 | 0.444 |  |  |
| $\mathbf{3}$ | 234 | 54 | 533 | 0.439 | 231 | 0.595 |  |  |

17:15-17:30

| Arm | Total Demand <br> (PCU/hr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput <br> $\mathbf{( P C U / h r )}$ | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 201 | 139 | 641 | 0.314 | 201 | 0.5 | 8.179 |  |
| $\mathbf{2}$ | 201 | 123 | 577 | 0.349 | 201 | 0.5 | 9.546 |  |
| $\mathbf{3}$ | 280 | 65 | 530 | 0.528 | 278 | 1.1 | 14.253 |  |

17:30-17:45

| Arm | Total Demand <br> (PCU/hr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput <br> $(\mathbf{P C U} / \mathbf{h r})$ | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 247 | 169 | 629 | 0.392 | 246 | 0.6 | 9.377 | A |
| $\mathbf{2}$ | 247 | 150 | 568 | 0.434 | 246 | 0.8 | 11.132 |  |
| $\mathbf{3}$ | 342 | 79 | 525 | 0.652 | 340 | 1.8 | 19.106 |  |

17:45-18:00

| Arm | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 247 | 171 | 629 | 0.392 | 247 | 0.6 | 9.422 | A |
| 2 | 247 | 151 | 568 | 0.434 | 247 | 0.8 | 11.199 | B |
| 3 | 342 | 79 | 525 | 0.652 | 342 | 1.8 | 19.625 | C |

18:00-18:15

| Arm | Total Demand <br> (PCU/hr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput <br> (PCU/hr) | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 201 | 141 | 640 | 0.315 | 202 | 0.5 |  |  |
| $\mathbf{2}$ | 201 | 124 | 577 | 0.349 | 202 | 0.5 | 9.623 |  |
| $\mathbf{3}$ | 280 | 65 | 529 | 0.528 | 282 | 1.2 |  |  |

## 18:15-18:30

| Arm | Total Demand <br> $(\mathbf{P C U} / \mathbf{h r})$ | Circulating flow <br> $(\mathbf{P C U} / \mathbf{h r})$ | Capacity <br> $(\mathbf{P C U} / \mathbf{h r})$ | RFC | Throughput <br> $(\mathbf{P C U} / \mathbf{h r})$ | End queue (PCU) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 169 | 117 | 649 | 0.260 | 169 | 0.4 | 7.512 |  |
| $\mathbf{2}$ | 169 | 103 | 584 | 0.289 | 169 | 0.4 | 8.689 | A |
| $\mathbf{3}$ | 234 | 54 | 533 | 0.440 | 236 | 0.8 | 12.177 | B |

Full Input Data And Results
Full Input Data And Results
User and Project Details

| Project: | Moylaragh Road - Harry Reynolds Road Roundabout Reconfiguration to <br> Signalised Junction |
| :--- | :--- |
| Title: | Harry Reynolds Road Pedestrian and Cycle Scheme <br> Location: <br> Client: |
| Moylaragh Road - Harry Reynolds Road Roundabout |  |
| Additional detail: |  |
| File name: | Junction 1.Isg3x County Council |
| Author: | Ben Holland |
| Company: <br> Address: | Atkins <br> 150 Lakeside Drive, Airside Business Park, Swords, Co. Dublin |

Network Layout Diagram



Phase Input Data

| Phase Name | Phase Type | Assoc. Phase | Street Min | Cont Min |
| :---: | :---: | :---: | :---: | :---: |
| A | Traffic |  | 7 | 7 |
| B | Traffic |  | 7 | 7 |
| C | Traffic |  | 7 | 7 |
| D | Pedestrian |  | 5 | 5 |
| E | Pedestrian |  | 5 | 5 |
| F | Pedestrian |  | 5 | 5 |
| G | Ind. Arrow | C | 5 | 5 |

Phase Intergreens Matrix


## Phases in Stage

| Stage No. | Phases in Stage |
| :---: | :--- |
| 1 | A C |
| 2 | C G |
| 3 | B |
| 4 | D E F |

## Stage Diagram



## Phase Delays

| Term. Stage | Start Stage | Phase | Type | Value | Cont value |
| :---: | :---: | :---: | :---: | :---: | :---: |
| There are no Phase Delays defined |  |  |  |  |  |

Prohibited Stage Change


Full Input Data And Results

## Give-Way Lane Input Data

## Junction: Unnamed Junction

There are no Opposed Lanes in this Junction

Full Input Data And Results
Lane Input Data

| Junction: Unnamed Junction |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Type | Phases | Start Disp. | End Disp. | Physical Length (PCU) | Sat Flow Type | Def User Saturation Flow (PCU/Hr) | Lane <br> Width <br> (m) | Gradient | Nearside Lane | Turns | Turning Radius (m) |
| 1/1 <br> (Harry Reynolds Road) | U | A | 2 | 3 | 60.0 | Geom | - | 3.50 | 0.00 | Y | $\underset{\text { Left }}{\text { Arm }}$ | 10.00 |
|  |  |  |  |  |  |  |  |  |  |  | Arm 6 Ahead | Inf |
| 2/1 (Harry Reynolds Road (south)) | U | B | 2 | 3 | 60.0 | Geom | - | 3.50 | 0.00 | Y | Arm 4 Right | 13.50 |
|  |  |  |  |  |  |  |  |  |  |  | Arm 6 Left | 9.50 |
| 3/1 <br> (Moylaragh Road) | U | C G | 2 | 3 | 60.0 | Geom | - | 3.50 | 0.00 | Y | Arm 4 Ahead | Inf |
|  |  |  |  |  |  |  |  |  |  |  | Arm 5 Right | 14.00 |
| 4/1 | U |  | 2 | 3 | 60.0 | Geom | - | 3.50 | 0.00 | Y |  |  |
| 5/1 | U |  | 2 | 3 | 60.0 | Geom | - | 3.50 | 0.00 | Y |  |  |
| 6/1 | U |  | 2 | 3 | 60.0 | Geom | - | 3.50 | 0.00 | Y |  |  |

## Traffic Flow Groups

| Flow Group | Start Time | End Time | Duration | Formula |
| :--- | :---: | :---: | :---: | :---: |
| 1: '2018 AM' | $08: 00$ | $09: 00$ | $01: 00$ |  |
| 2: '2018 PM' | $17: 00$ | $18: 00$ | $01: 00$ |  |

Scenario 1: 'AM Peak' (FG1: '2018 AM', Plan 1: 'Network Control Plan 1')
Traffic Flows, Desired
Desired Flow :

|  | Destination |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | A | B | C | Tot. |  |
|  | A | 0 | 218 | 88 | 306 |  |
|  | B | 121 | 0 | 82 | 203 |  |
|  | C | 101 | 276 | 0 | 377 |  |
|  | Tot. | 222 | 494 | 170 | 886 |  |

## Traffic Lane Flows

| Lane | Scenario 1: <br> AM Peak |
| :---: | :---: |
| Junction: Unnamed Junction |  |
| $1 / 1$ | 306 |
| $2 / 1$ | 203 |
| $3 / 1$ | 377 |
| $4 / 1$ | 222 |
| $5 / 1$ | 494 |
| $6 / 1$ | 170 |

Full Input Data And Results

## Lane Saturation Flows

| Junction: Unnamed Junction |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| 1/1 <br> (Harry Reynolds Road) | 3.50 | 0.00 | Y | Arm 5 Left | 10.00 | 71.2 \% | 1775 | 1775 |
|  |  |  |  | Arm 6 Ahead | Inf | 28.8 \% |  |  |
| $2 / 1$(Harry Reynolds Road (south)) | 3.50 | 0.00 | Y | Arm 4 Right | 13.50 | 59.6 \% | 1739 | 1739 |
|  |  |  |  | Arm 6 Left | 9.50 | 40.4 \% |  |  |
| 3/1 <br> (Moylaragh Road) | 3.50 | 0.00 | Y | Arm 4 Ahead | Inf | 26.8 \% | 1822 | 1822 |
|  |  |  |  | Arm 5 Right | 14.00 | 73.2 \% |  |  |
| 4/1 | 3.50 | 0.00 | Y |  |  |  | 1965 | 1965 |
| 5/1 | 3.50 | 0.00 | Y |  |  |  | 1965 | 1965 |
| 6/1 | 3.50 | 0.00 | Y |  |  |  | 1965 | 1965 |

Scenario 2: 'PM Peak' (FG2: '2018 PM', Plan 1: 'Network Control Plan 1')
Traffic Flows, Desired
Desired Flow :

|  | Destination |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | A | B | C | Tot. |  |
|  | A | 0 | 152 | 72 | 224 |  |
|  | B | 155 | 0 | 156 | 311 |  |
|  | C | 87 | 137 | 0 | 224 |  |
|  | Tot. | 242 | 289 | 228 | 759 |  |

## Traffic Lane Flows

| Lane | Scenario 2: <br> PM Peak |
| :---: | :---: |
| Junction: Unnamed Junction |  |
| $1 / 1$ | 224 |
| $2 / 1$ | 311 |
| $3 / 1$ | 224 |
| $4 / 1$ | 242 |
| $5 / 1$ | 289 |
| $6 / 1$ | 228 |

Full Input Data And Results
Lane Saturation Flows

| Junction: Unnamed Junction |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane | Lane Width (m) | Gradient | Nearside Lane | Allowed Turns | Turning Radius (m) | Turning Prop. | Sat Flow (PCU/Hr) | Flared Sat Flow (PCU/Hr) |
| $1 / 1$(Harry Reynolds Road) | 3.50 | 0.00 | Y | Arm 5 Left | 10.00 | 67.9 \% | 1783 | 1783 |
|  |  |  |  | Arm 6 Ahead | Inf | 32.1 \% |  |  |
| $2 / 1$(Harry Reynolds Road (south)) | 3.50 | 0.00 | Y | Arm 4 Right | 13.50 | 49.8 \% | 1732 | 1732 |
|  |  |  |  | Arm 6 Left | 9.50 | 50.2 \% |  |  |
| 3/1 (Moylaragh Road) | 3.50 | 0.00 | Y | Arm 4 Ahead | Inf | 38.8 \% | 1844 | 1844 |
|  |  |  |  | Arm 5 Right | 14.00 | 61.2 \% |  |  |
| 4/1 | 3.50 | 0.00 | Y |  |  |  | 1965 | 1965 |
| 5/1 | 3.50 | 0.00 | Y |  |  |  | 1965 | 1965 |
| 6/1 | 3.50 | 0.00 | Y |  |  |  | 1965 | 1965 |

Scenario 1: 'AM Peak' (FG1: '2018 AM', Plan 1: 'Network Control Plan 1') Stage Sequence Diagram


Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ |
| :---: | :---: | :---: | :---: | :---: |
| Duration | 51 | 5 | 34 | 5 |
| Change Point | 0 | 59 | 69 | 108 |

Signal Timings Diagram



Full Input Data And Results

## Network Results

| Item | Lane <br> Description | $\begin{aligned} & \text { Lane } \\ & \text { Type } \end{aligned}$ | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow <br> Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | $\begin{aligned} & \text { Deg Sat } \\ & \text { (\%) } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Harry Reynolds Road Pedestrian and Cycle Scheme | - | - | N/A | - | - |  | - | - | - | - | - | - | 40.0\% |
| Unnamed Junction | - | - | N/A | - | - |  | - | - | - | - | - | - | 40.0\% |
| 1/1 | Harry Reynolds Road Left Ahead | U | N/A | N/A | A |  | 1 | 51 | - | 306 | 1775 | 769 | 39.8\% |
| 2/1 | Harry Reynolds Road (south) Right Left | U | N/A | N/A | B |  | 1 | 34 | - | 203 | 1739 | 507 | 40.0\% |
| 3/1 | Moylaragh Road Ahead Right | U | N/A | N/A | C | G | 1 | 61 | 5 | 377 | 1822 | 941 | 40.0\% |
| 4/1 |  | U | N/A | N/A | - |  | - | - | - | 222 | 1965 | 1965 | 11.3\% |
| 5/1 |  | u | N/A | N/A | - |  | - | - | - | 494 | 1965 | 1965 | 25.1\% |
| 6/1 |  | U | N/A | N/A | - |  | - | - | - | 170 | 1965 | 1965 | 8.7\% |
| Ped Link: P1 | Unnamed Ped Link | - | - | - |  |  | 0 | 0 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P2 | Unnamed Ped Link | - | - | - |  |  | 0 | 0 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P3 | Unnamed Ped Link | - | - | - |  |  | 0 | 0 | - | 0 | - | 0 | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + <br> Oversat <br> Delay <br> (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean <br> Max <br> Queue <br> (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Harry Reynolds Road Pedestrian and Cycle Scheme | - | - | 0 | 0 | 0 | 5.9 | 1.3 | 0.0 | 7.2 | - | - | - | - |
| Unnamed Junction | - | - | 0 | 0 | 0 | 5.9 | 1.3 | 0.0 | 7.2 | - | - | - | - |
| 1/1 | 306 | 306 | - | - | - | 2.0 | 0.3 | - | 2.3 | 27.2 | 7.0 | 0.3 | 7.3 |
| 2/1 | 203 | 203 | - | - | - | 1.9 | 0.3 | - | 2.3 | 40.0 | 5.4 | 0.3 | 5.7 |
| 3/1 | 377 | 377 | - | - | - | 1.9 | 0.3 | - | 2.2 | 20.9 | 7.6 | 0.3 | 8.0 |
| 4/1 | 222 | 222 | - | - | - | 0.0 | 0.1 | - | 0.1 | 1.0 | 0.0 | 0.1 | 0.1 |
| 5/1 | 494 | 494 | - | - | - | 0.1 | 0.2 | - | 0.3 | 2.3 | 10.4 | 0.2 | 10.6 |
| 6/1 | 170 | 170 | - | - | - | 0.0 | 0.0 | - | 0.0 | 1.0 | 0.0 | 0.0 | 0.0 |
| Ped Link: P1 | 0 | 0 | - | - | - | - | - | - | Inf | Inf | - | - | Inf |
| Ped Link: P2 | 0 | 0 | - | - | - | - | - | - | Inf | Inf | - | - | Inf |
| Ped Link: P3 | 0 | 0 | - | - | - | - | - | - | Inf | Inf | - | - | Inf |
| C1 |  |  | PRC for Signalled Lanes (\%): <br> PRC Over All Lanes (\%): |  | $\begin{align*} & 124.7  \tag{Tc}\\ & 1247 \end{align*}$ |    <br> Total Delay for Signalled Lanes $(\mathrm{pcuHr}):$ 6.75  <br> Total Delay Over All Lanes(pcuHr): 7.18  |  |  | Cycle Time (s): 120 |  |  |  |  |

Full Input Data And Results
Scenario 2: 'PM Peak' (FG2: '2018 PM', Plan 1: 'Network Control Plan 1')
Stage Sequence Diagram


## Stage Timings

| Stage | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ |
| :---: | :---: | :---: | :---: | :---: |
| Duration | 35 | 5 | 50 | 5 |
| Change Point | 0 | 43 | 53 | 108 |

Signal Timings Diagram


## Network Layout Diagram



Full Input Data And Results

## Network Results

| Item | Lane Description | $\begin{aligned} & \text { Lane } \\ & \text { Type } \end{aligned}$ | Controller Stream | Position In Filtered Route | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat <br> (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Harry Reynolds Road Pedestrian and Cycle Scheme | - | - | N/A | - | - |  | - | - | - | - | - | - | 42.2\% |
| Unnamed Junction | - | - | N/A | - | - |  | - | - | - | - | - | - | 42.2\% |
| 1/1 | Harry Reynolds Road Left Ahead | U | N/A | N/A | A |  | 1 | 35 | - | 224 | 1783 | 535 | 41.9\% |
| 2/1 | Harry Reynolds Road (south) Right Left | U | N/A | N/A | B |  | 1 | 50 | - | 311 | 1732 | 736 | 42.2\% |
| 3/1 | Moylaragh Road Ahead Right | U | N/A | N/A | C | G | 1 | 45 | 5 | 224 | 1844 | 707 | 31.7\% |
| 4/1 |  | U | N/A | N/A | - |  | - | - | - | 242 | 1965 | 1965 | 12.3\% |
| 5/1 |  | U | N/A | N/A | - |  | - | - | - | 289 | 1965 | 1965 | 14.7\% |
| 6/1 |  | U | N/A | N/A | - |  | - | - | - | 228 | 1965 | 1965 | 11.6\% |
| Ped Link: P1 | Unnamed Ped Link | - | - | - |  |  | 0 | 0 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P2 | Unnamed Ped Link | - | - | - |  |  | 0 | 0 | - | 0 | - | 0 | 0.0\% |
| Ped Link: P3 | Unnamed Ped Link | - | - | - |  |  | 0 | 0 | - | 0 | - | 0 | 0.0\% |

Full Input Data And Results

| Item | Arriving (pcu) | Leaving (pcu) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Uniform Delay (pcuHr) | Rand + <br> Oversat <br> Delay <br> (pcuHr) | Storage Area Uniform Delay (pcuHr) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Max. Back of Uniform Queue (pcu) | Rand + Oversat Queue (pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Harry Reynolds Road Pedestrian and Cycle Scheme | - | - | 0 | 0 | 0 | 5.8 | 1.2 | 0.0 | 7.0 | - | - | - | - |
| Unnamed Junction | - | - | 0 | 0 | 0 | 5.8 | 1.2 | 0.0 | 7.0 | - | - | - | - |
| 1/1 | 224 | 224 | - | - | - | 2.1 | 0.4 | - | 2.5 | 39.4 | 6.0 | 0.4 | 6.3 |
| 2/1 | 311 | 311 | - | - | - | 2.1 | 0.4 | - | 2.5 | 28.4 | 7.3 | 0.4 | 7.6 |
| 3/1 | 224 | 224 | - | - | - | 1.6 | 0.2 | - | 1.8 | 29.7 | 5.2 | 0.2 | 5.5 |
| 4/1 | 242 | 242 | - | - | - | 0.0 | 0.1 | - | 0.1 | 1.0 | 0.0 | 0.1 | 0.1 |
| 5/1 | 289 | 289 | - | - | - | 0.0 | 0.1 | - | 0.1 | 1.4 | 5.7 | 0.1 | 5.8 |
| 6/1 | 228 | 228 | - | - | - | 0.0 | 0.1 | - | 0.1 | 1.0 | 0.0 | 0.1 | 0.1 |
| Ped Link: P1 | 0 | 0 | - | - | - | - | - | - | Inf | Inf | - | - | Inf |
| Ped Link: P2 | 0 | 0 | - | - | - | - | - | - | Inf | Inf | - | - | Inf |
| Ped Link: P3 | 0 | 0 | - | - | - | - | - | - | Inf | Inf | - | - | Inf |
| C1 |  |  | $\begin{array}{cc} \text { PRC for Signalled Lanes (\%): } & 11 \\ \text { PRC Over All Lanes (\%): } & 11 \end{array}$ |  | Total Delay for Signalled Lanes (pcuHr): 6.75 <br> Total Delay Over All Lanes $(\mathrm{pcuHr}):$ 7.00 |  |  |  | Cycle Time (s): 120 |  |  |  |  |

## Appendix B. Bridge Options

As part of the wider Harry Reynolds Road scheme it is proposed to upgrade 2 no. footbridges located in the public park to the northeast of Harry Reynolds Road. These structures provide pedestrian access between Vauxhall Street and Clonard Court across the Bracken River. The structures will need to be widened or replaced to accommodate the proposed 4 m wide pedestrian \& cyclist facilities through the public park as part of the overall scheme.

A third structure is also present to the north of these footbridges. This footbridge provides pedestrian access between Vauxhall Street and Clonard Street. This structure has been omitted from the initial study but may be considered at a later stage.

## Description of Existing Structures

The 2 no. footbridges are of similar construction, consisting of a reinforced concrete substructure with the superstructure comprising galvanised steel sheeting and transverse beams supported on 2 no. longitudinal reinforced concrete beams. The existing parapets are 1.125 m high painted steel parapets with a mesh infill. Both structures have a 450 mm diameter concrete outfall through the west abutment, discharging drainage water into the watercourse via a concrete apron.

The south structure has a span of 5.82 m and an out-to-out width of 2.2 m . The central structure has a span of 7.2 m and an out-to-out width of 2.2 m .


CW from top left: south elevation of the south footbridge, south elevation of the central footbridge, general view of the soffit of the steel decking from below the central footbridge, general view across the south footbridge.

## Options Considered

The 3no. options considered to accommodate the upgraded 4 m wide pedestrian \& cyclist facilities are as follows:

1. Install new cyclist only structures adjacent to the existing structures.
2. Widen the existing structures to meet the required 4 m width.
3. Replace the existing structures with new 4 m wide structures.

## Evaluation of Options

The 3no. options outlined in the section above have been evaluated for numerous criteria including; complexity, construction and whole life costs, environmental impact, buildability, aesthetics, and health and safety.

## Option 1 - New cyclist only structures

The existing structures would be retained in their current form for pedestrian use only. 2 no. new 2 m wide structures would be constructed adjacent to facilitate the passage of cyclists across the watercourse.

The construction of new structures for only cyclist use has the benefit of retaining the use of the existing structure during the construction phase. However, this option would impact the surrounding riverbanks and watercourse, doubling the footprint area of the existing crossings. The cost and duration of the construction phase would be excessive compared to widening the existing structures, with whole life costs also increasing due to the maintenance requirements of the additional structures. Operational issues may occur with both pedestrians and cyclists using a single structure, selecting the shortest route across the Bracken River.

## Option 2 - Widen the existing structures

The existing structures would be widened from 2 m to 4 m in order to facilitate both the pedestrian and cyclist facilities. Due to the modular nature of the superstructure the existing steel decking could be removed and replaced with a wider steel deck, cantilevering 1 m either side of the existing reinforced concrete longitudinal beams.

The widening of the existing structures by replacing the steel decking offers a low cost and low complexity solution, subject to the structural capacity of the existing reinforced concrete beams and substructure being capable of supporting the increased deck area. The replacement steel decks would be manufactured off site and lifted into position, limiting works on site and reducing site health and safety risks. The overall cost of the structures would be benefitted by retaining the existing substructures and primary deck elements. Minor land take from the adjacent Balbriggan Enterprise and Training Centre would be required at the south west corner of the structure to accommodate a 4 m structure width.

## Option 3 - Replacement of the existing structures

The existing structure would be demolished to foundation level and replaced with a new structure to accommodate the 4 m wide pedestrian and cyclist facilities.

The replacement of the existing structures would provide the most aesthetic solution but also the most expensive construction cost. The duration of the construction phase would also be increased due to the demolition of the existing structures, restricting public access for a longer period. Whole life costs of the structure would be favourable compared to the other options, as the new construction could be designed for improved durability. The demolition/construction would cause a short term negative impact on the surrounding environment, requiring both the dewatering of the watercourse and the temporary diversion of the outfall pipes through the abutments.

## Conclusion \& Recommendation

While all 3no. options evaluated are feasible, the proposed option at this initial stage is Option 2 - Widen the existing structures. Option 2 makes best use of the existing structures and therefore has the lowest construction cost and environmental impact. The use of the existing structures is subject to structural assessments confirming the additional capacity required for the widened steel decks. Minor land take from the southeast corner of the Balbriggan Enterprise and Training Centre will also be required to facilitate the widening of the south structure.

## Appendix C. Permeability Links



Image 1


Image 2


Image 3


Image 4


Image 5


Image 6


## Appendix D. Preferred Route



## Contact name

Atkins company name
Office address

## Email <br> Telephone <br> Direct telephone <br> Fax

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[^0]:    The slope and intercept shown above include any corrections and adjustments.

