



Flood Risk Assessment

FINGAL COUNTY COUNCIL - Residential Development at Mooretown, Swords, Co. Dublin

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Comments



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1. Introduction

This Flood Risk Assessment has been prepared by Waterman Moylan as part of the documentation in support of a proposed residential development in Mooretown, Swords, Co. Dublin.

The proposed development forms part of the Mooretown lands in Swords, which were previously informed by the Oldtown-Mooretown Local Area Plan 2010-2016 (as extended to 2020) and are now subject to the Fingal development Plan 2020. This phase of developing the lands has been designed and proposed within the spirit of the LAP and in compliance with the Fingal Development Plan.

1.1 Flood Risk Assessment: Statement of Design Consistency

This Flood Risk Assessment has been carried out in accordance with the *DEHLG/OPW Guidelines on the Planning Process and Flood Risk Management* published in November 2009. This assessment identifies the risk of flooding at the site from various sources and sets out possible mitigation measures against the potential risks of flooding. Sources of possible flooding include coastal, fluvial, pluvial (direct heavy rain), groundwater and human/mechanical errors. This report provides an assessment of the subject site for flood risk purposes only.

1.2 Site Description

The subject site is in Mooretown, Swords, Co. Dublin, to the south of the Rathbeale Road, as shown in the Figure below:



Figure 1 | Site Location (Source: Google Earth)

The site is part of the residentially zoned Oldtown–Mooretown lands, as shown in the Figure 2 extract below, taken from the Fingal Development Plan 2023-2029 Zoning Map – Sheet 8:



Figure 2 | Extract of the Fingal Development Plan 2023-2029 Zoning Map - Sheet 8

The Oldtown–Mooretown lands are located at the western development edge of Swords, within the catchment of the Broadmeadow River. The lands cover an area of approximately 111 hectares.

The Oldtown–Mooretown lands are divided by the Rathbeale Road, with Oldtown lands to the north (c. 50 Ha) and Mooretown to the south (c. 61 Ha). This Local Authority Own Housing development, which forms Phase 1 of the Mooretown Fingal Council Mooretown development, is located on the eastern side of the "Mooretown" Lands as indicated in *Figure 2*. The "Mooretown" Lands to the west of the subject lands are under private development.

The Phase 1 development site is currently a greenfield, ~9.35 Ha in size, and is located 2km west of Swords town centre.

The subject site is bounded to the east by the existing Cianlea housing development, to the north by the recently upgraded Rathbeale Road, to the west by the completed Mooretown School Campus and the adjacent privately developed Mooretown Lands.

The site lies withing the catchment of the Broadmeadow River which outfalls to the Malahide estuary. The estuary is a Special Protection Area (SPA), a candidate Special Area of Conservation (cSAC) a proposed National heritage Area (pNHA) and a RAMSAR site.

The full Mooretown site is drained by surface ditches/streams which in turn flow to the Mooretown Mill Stream, in the northern third of the subject site. This Mill Stream flows north to Rathbeale Road where it is culverted by 1200mmØ pipes which traverses the eastern boundary of the Oldtown lands before ultimately discharging to the Broadmeadow River.

Neither the Mooretown Lands nor the Broadmeadow River are part of the SPA or SAC site, however, any development immediately upstream is required to maintain, or improve the quality of surface water to status objectives, as set out in the Water Framework Directive (WFD). These requirements are in place in order to protect and enhance the status of the aquatic ecosystems of the SAC or SPA site. This will require the implementation of SuDS, which are intended to be utilised as part of the development.

1.3 Proposed Development

The subject site for the proposed Phase 1 development is located on Lands at Mooretown, at the northwestern edge of the town of Swords, North County Dublin, approx. 2km from the town centre. The proposed Phase 1 development is 9.35 Ha in size, and seeks the construction of:

- A total of 274 no. residential units including 187 no. houses, 37 no. duplex, and 50 no. apartment units, ranging from 2 5 storeys in height, in a mixed tenure development (18 no. 1-beds, 109 no. 2-beds, 128 no. 3-beds and 19 no. 4-beds)
- b. Landscaping works including the provision of Class 2 Open Space of c.18,065 sq.m including riparian corridors, 8 no. pocket parks with a total area of c.2,900 sq.m, and new pedestrian and cycle connections into neighbouring residential lands to the west and connecting to the existing school access road along the western boundary.
- c. A total of 415 no. car parking spaces consisting of 357 no. long-stay resident spaces, 58 no. shortstay and visitor spaces.
- d. A total of 1,143 no. bicycle parking spaces consisting of 1,117 no. long-stay resident spaces and 26 no. short-stay visitor spaces.
- e. Associated site and infrastructural works including the provision of foul and surface water drainage and associated connections, Sustainable Urban Drainage Systems including permeable paving, greens roofs, bio-retention planting and below ground tank storage.
- f. The proposed development includes for all site enabling and development works, landscaping works, plant, PV panels, storage, boundary treatments, ESB substations, lighting, servicing, signage, and all site development works above and below ground.

The detailed breakdown of the proposed residential scheme is as follows:

Typology	1Bed	2Bed	3Bed	4Bed	Total
Houses		54	114	19	187
Duplexes	14	20	3		37
Apartments	4	35	11		50
Total	18	109	128	19	274

Table 1 | Schedule of Accommodation

1.4 Background to the Report

This Flood Risk Assessment report follows the guidelines set out in the *DEHLG/OPW Guidelines on the Planning Process and Flood Risk Management* published in November 2009. The components to be considered in the identification and assessment of flood risk are as per Table A1 of the above guidelines:

- Tidal flooding from high sea levels
- Fluvial flooding from water courses
- Pluvial flooding from rainfall / surface water
- Groundwater flooding from springs / raised groundwater
- Human/mechanical error flooding due to human or mechanical error

Each component will be investigated from a Source, Pathway and Receptor perspective, followed by an assessment of the likelihood of a flood occurring and the possible consequences.

1.4.1 Assessing Likelihood

The likelihood of flooding falls into three categories of low, moderate and high, which are described in the OPW Guidelines as follows:

Flood Risk	Likelihood: % chance of occurring in a year			
Components	Low Moderate		High	
Tidal	Probability < 0.1%	0.5% > Probability > 0.1%	Probability > 0.5%	
Fluvial	Probability < 0.1%	1% > Probability > 0.1%	Probability > 1%	
Pluvial	Probability < 0.1%	1% > Probability > 0.1%	Probability > 1%	

Table 2 | From Table A1 of "DEHLG/OPW Guidelines on the Planning Process and Flood Management"

For groundwater and human/mechanical error, the limits of probability are not defined and therefore professional judgment is used. However, the likelihood of flooding is still categorized as low, moderate and high for these components.

From consideration of the likelihoods and the possible consequences a risk is evaluated. Should such a risk exist, mitigation measures will be explored, and the residual risks assessed.

1.4.2 Assessing Consequence

There is not a defined method used to quantify a value for the consequences of a flooding event. Therefore, in order to determine a value for the consequences of a flooding event, the elements likely to be adversely affected by such flooding will be assessed, with the likely damage being stated, and professional judgement will be used in order to determine a value for consequences. Consequences will also be categorized as low, moderate, and high.

1.4.3 Assessing Risk

Based on the determined 'likelihood' and 'consequences' values of a flood event, the following 3x3 Risk Matrix will then be referenced to determine the overall risk of a flood event.

		Consequences		
		Low	Moderate	High
Likelihood	Low	Extremely Low Risk	Low Risk	Moderate Risk
	Moderate	Low Risk	Moderate Risk	High Risk
	High	Moderate Risk	High Risk	Extremely High Risk

Table 3 | 3x3 Risk Matrix

2. Sequential Test

2.1 General

A sequential approach to planning is a key tool in ensuring that a development, particularly any new development, is first and foremost directed towards land that is at low risk of flooding. The sequential approach is set out in "The Planning System and Flood Risk Management Guidelines for Planning Authorities, 2009" and is referred to in the Strategic Flood Risk Assessment for the Fingal Development Plan 2017-2023.

The sequential approach is illustrated in the Figure below:



Figure 3 | Sequential Approach

2.2 Establish Flood Zone

The first step of the sequential test is to establish the flood zone within which the site lies.

The subject site is in Flood Zone C, as it is outside the 1-in-1,000-year flood zone for both tidal and fluvial flooding – refer to Sections 3 and 4, below, for further information on tidal and fluvial flooding, respectively.

2.3 Establish Vulnerability Class

The next step is to establish the vulnerability class of the proposal. The Table below, taken from the OPW's "Planning and Flood Risk Management Guidelines for Planning Authorities, 2009" document, lists the vulnerability classes assigned to various land uses and types of development:

Vulnerability Class Land Uses and Types of Development which include*:		
	Garda, ambulance and fire stations and command centres required to be operational during flooding; Hospitals;	
	Emergency access and egress points;	
	Schools;	
Highly yulgoroblo	Dwelling houses, student halls of residence and hostels;	
development (including	Residential institutions such as residential care homes, children's homes and social services homes;	
	Caravans and mobile home parks;	
	Dwelling houses designed, constructed or adapted for the elderly or other people with impaired mobility; and	
	Essential infrastructure, such as primary transport and utilities distribution, including electricity generating power stations and sub-stations, water and sewage treatment, and potential significant sources of pollution (SEVESO sites, IPPC sites, etc.) in the event of flooding.	
	Buildings used for: retail, leisure, warehousing, commercial, industrial and non-residential institutions;	
	Land and buildings used for holiday or short-let caravans and campong, subject to specific warning and evacuation plans;	
development	Land and buildings used for agriculture and forestry;	
	Waste treatment (except landfill and hazardous waste);	
	Mineral working and processing; and	
	Local transport infrastructure.	
	Flood control infrastructure;	
	Docks, marinas and wharves;	
	Navigation facilities;	
Water competible	Ship building, repairing and dismantling, dockside fish processing and refrigeration and compatible activities requiring a waterside location;	
development	Water-based recreation and tourism (excluding sleeping accommodation);	
	Lifeguard and coastguard stations;	
	Amenity open space, outdoor sports and recreation and essential facilities such as changing rooms; and	
	Essential ancillary sleeping or residential accommodation for staff required by uses in this category (subject to a specific warning and evacuation plan).	

*Uses not listed here should be considered on their own merits

 Table 4 | Vulnerability Classification of Different Types of Development

The proposed development is a residential development and is therefore considered highly vulnerable development.

2.4 Assess Justification Test Requirement

The Table below outlines the matrix of vulnerability based on the Flood Zone:

Description	Flood Zone A	Flood Zone B	Flood Zone C
Highly vulnerable development (including essential infrastructure)	Justification Test	Justification Test	Appropriate
Less vulnerable development	Justification Test	Appropriate	Appropriate
Water-compatible development	Appropriate	Appropriate	Appropriate

Table 5 | Vulnerability Matrix

Given that the subject site is within Flood Zone C, no justification test is required for the development, and development is considered appropriate.

3. Tidal

3.1 Source

Tidal flooding occurs when normally dry, low-lying land is flooded by seawater. The extent of tidal flooding is a function of the elevation inland flood waters penetrate, which is controlled by the topography of the coastal land exposed to flooding.

3.2 Pathway

The site is approximately 3.5km west of the nearest coastline at the Malahide Estuary, as shown in *Figure 4*. This figure is extracted from the OPW's flood information portal, shows that the site is not at risk of coastal flooding for even the 1-in-1,000 year flood event. The Dublin Coastal Protection Project indicates that the 2002 high tide event reached 2.95m OD Malin. The lowest existing ground level on site is approx. 29m, well above the historic high tide event.



Figure 4 | Extract from the FEM FRAMS Tidal Flood Extents Map

High probability flood events, as shown in the above map, are defined as having approximately a 1-in-10 chance of occurring or being exceeded in any given year (10% Annual Exceedance Probability), medium probability flood events are defined as having an AEP of 0.5% (1-in-200 year storm), while low probability events are defined having an AEP of 0.1% (1-in-1,000 year storm). The map indicates that the subject development is not at risk of flooding for the 1-in-1,000 year event.

Given that the site is located 3.5 kilometres inland from the Irish Sea, that there is at least a 26m level difference between the proposed buildings and the high tide and given that the site is outside of the 1-in-1,000 year flood plain, it is evident that a pathway does not exist between the source and the receptor. A risk from tidal flooding is therefore extremely low and no flood mitigation measures need to be implemented in this regard.

4. Fluvial

4.1 Source

Fluvial flooding occurs when a river / water course's flow exceeds its capacity, typically following excessive rainfall, though it can also result from other causes such as heavy snow melt and ice jams.

4.2 Pathway

The subject site is located within the Broadmeadow River catchment.

The Fingal East Meath Flood Risk Assessment and Management Study (FEM FRAMS) maps, available on the OPW's National Flood Information Portal and extracted below, shows that none of the subject site falls within the 0.1% AEP (1-in-1,000 year) flood plain.

The nearest node point, reference number: 4Bae1386, located on a tributary ditch leading to the Mill Stream, will have a 1-in-1,000 year flood event water height of 25.75m. This node point is on the eastern boundary of the subject site. There is no identified risk of flood from the Mill Stream through the subject Mooretown lands. The Mill Stream does not have any out of bank flow indicated on the CFRAM model for a 1 in 1000-year event, as indicated in below Figure 4. The nearest top of bank level of the Mill Stream is more than 2m below the closest Finished Floor Level within Phase 1, affording more than adequate freeboard.



Figure 5 | Extract from the FEM FRAMS Fluvial Flood Extents Map BRO/HPW/EXT/CURS/009

4.3 Likelihood

Given that the site is outside of the 1-in-1,000-year flood plain, the likelihood of fluvial flooding is low.

4.4 Consequence

The consequence of fluvial flooding would be some minor inundation to open spaces. Therefore, the consequences of fluvial flooding occurring at the proposed development is considered low.

4.5 Risk

There is an extremely low risk of fluvial flooding as the likelihood is low and the consequence is low.

4.6 Flood Risk Management

The development has been designed to provide overland flood routing through the road network and ultimately to the Mill Stream, which ultimately drains to the Broadmeadow River as described in Section 1.1. The overland flood routing is shown in full on drawing number: P1500, which is extracted overleaf in *Figure 6*.

The proposed development has designed finished floor levels generally over 200mm above the local road network to minimise the risk of flooding from overland flows.

There are no localised low points occurring in the road network. The result of this is a significant reduction in the risk of flooding for the subject site.

To minimise the risk of downstream flooding, surface water outflow from the site is limited to its equivalent green-field run-off rate via a flow control manhole. The surface water outfall detention basins and ponds have been designed to attenuate flood volumes for a 1 in 100-year event minimising the risk of downstream flooding.



Figure 6 | Overland Flood Route Extract

4.7 Residual Risk

The residual risk of fluvial flooding is considered extremely low.

5. Pluvial

5.1 Source

Pluvial flooding occurs when heavy rainfall creates a flood event independent of an overflowing water body. Pluvial flooding can happen in any urban area, including higher elevation areas that lie above coastal and river floodplains.

5.2 Pathway & Receptors

During periods of extreme prolonged rainfall, pluvial flooding may occur through the following pathways:

	Pathway	Receptor
1	Surcharging of the proposed internal drainage systems during heavy rain events leading to internal flooding	Proposed development – properties and roads
2	Surcharging from the existing surrounding drainage system leading to flooding within the subject site by surcharging surface water pipes	Proposed development – properties and roads
3	Surface water discharging from the subject site to the existing drainage network leading to downstream flooding	Downstream properties and roads
4	Overland flooding from surrounding areas flowing onto the subject site	Proposed development – properties and roads
5	Overland flooding from the subject site flowing onto surrounding areas	Downstream properties and roads

Table 6 | Pathways and Receptors

5.3 Likelihood

The likelihood of each of the 5 pathway types are addressed individually as follows:

5.3.1 Surcharging of the proposed on-site drainage systems:

The proposed on-site surface water drainage sewers have been designed to accommodate flows from a 5year return event, which indicates that on average the internal system may surcharge during rainfall events with a return period in excess of five years. Therefore, the likelihood surcharging of the on-site drainage system is considered high.

5.3.2 Surcharging from the existing surrounding drainage system:

The OPW's on-line portal was again consulted to ascertain the details of any local historic flood events. *Figure 7* overleaf, shows that there is no record of a previous flood event at the subject site, with the nearest historic flood event occurring approx. 1.3km away to the east. Information on this flood event shows that it occurred in November 1982. The report notes that 64% of the average monthly rainfall volume for November fell within a 12-hour period, which was compounded by heavy rainfalls in the preceding days which had led to ground saturation. This specific flood event was of a residential unit's rear garden and was due to a blockage of the local surface water drainage network. Drainage engineers attended the location and remediated the blockage. No flood events at this location have been recorded since.



Figure 7 | Local Flood Event History Extracted from OPW's National Flood Hazard Maps

With no history of flooding in the area due to surcharging, the likelihood of such flooding occurring is considered low.

5.3.3 Surface water discharge from the subject site:

Due to the increase in hard standing area as a result of the proposed development, there is an increased likelihood of surface water discharge from the site leading to downstream flooding. As such, the likelihood can be considered moderate.

5.3.4 Overland flooding from surrounding areas:

With no recorded flood events in the immediate area that could have an impact on the subject site, as per the OPW records referred to above, it is considered that there is a low likelihood of flooding from surrounding areas.

5.3.5 Overland flooding from the subject site:

Due to the increase in hard standing area as a result of the proposed development, there is an increased likelihood of overland flooding from the site leading to downstream flooding. As such, the likelihood can be considered moderate.

5.4 Consequence

Surface water flooding would result in damage to roads and landscaped areas and could impact the ground floor levels of buildings. The consequences of pluvial flooding are considered moderate.

5.5 Risk

The risk of each of the 5 pathway types is addressed individually as follows:

5.5.1 Surcharging of the proposed on-site drainage systems:

With a high likelihood and moderate consequence of flooding the site from surcharging the on-site drainage system, the resultant risk is high.

5.5.2 Surcharging from the existing surrounding drainage system:

With a low likelihood and moderate consequence of flooding the site from the existing surface water network, the resultant risk is low.

5.5.3 Surface water discharge from the subject site:

With a moderate likelihood and moderate consequence of surface water discharge from the subject site, the resultant risk is moderate.

5.5.4 Overland flooding from surrounding areas:

With a low likelihood and moderate consequence of overland flooding from the surrounding areas, the resultant risk is low.

5.5.5 Overland flooding from the subject site:

With a moderate likelihood and moderate consequence of overland flooding from the subject site, the resultant risk is moderate.

5.6 Flood Risk Management

The following are flood risk management strategies proposed to minimise the risk of pluvial flooding for each risk:

5.6.1 Surcharging of the proposed on-site drainage systems:

The risk of flooding is minimised with adequate sizing of the on-site surface water network and SuDS devices. Open grassed areas with low level planting will ensure that these areas act as soft scape and will significantly slow down and reduce the amount of surface water runoff from the site. Permeable paving in private driveways and parking courts and filter drains around the perimeter of the open spaces will provide some treatment volume, with underlying perforated pipes connecting to the storm water sewer network.

These proposed source and site control devices will intercept and slow down the rate of runoff from the site to the on-site drainage system, reducing the risk of surcharging.

Furthermore, a hydro-brake for each catchment will limit runoff to the equivalent greenfield rate. Excess storm water from the main catchment is to be attenuated in the dry detention basin/ponds with sufficient volume for the 1-in-100-year storm (accounting for a 20% increase due to climate change), to limit the runoff from the site and minimise the discharge rate into receiving waters. Please refer to the Engineering

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Assessment Report, submitted under a separate cover, which details in full the Surface Water drainage strategy for the overall Mooretown Development.

As a result of these proposed measures, the likelihood of surcharging of the proposed on-site drainage systems is low.

5.6.2 Surcharging from the existing surrounding drainage system:

The risk of flooding due to surcharging of the existing surface water network is minimised with overland flood routing (refer to the Overland Flood Routing figure in Section 4.6 above) towards the Mill Stream and towards the dry detention basin and attenuation pond. The risk to the surrounding buildings is mitigated by setting finished floor levels at least 200mm above the adjacent road channel line.

5.6.3 Surface water discharge from the subject site:

Surface water discharge from the subject site is intercepted and slowed down through the use of source control devices, as described in Section 5.6.1 above, minimising the risk of pluvial flooding from the subject site. Sufficient attenuation storage is provided for the 1-in-100 year storm, accounting for a 20% increase due to climate change.

5.6.4 Overland flooding from surrounding areas:

The risk from overland flooding from surrounding areas is low. Overland flood routing and raised finished floor levels will provide protection for the proposed buildings, as described in Section 5.6.2 above.

5.6.5 Overland flooding from the subject site:

The risk of overland flooding from the subject site is minimised by providing SuDS features to intercept and slow down the rate of runoff from the site to the existing surface water sewer system, as described in Section 5.6.1 above. Sufficient attenuation is provided for the 1-in-100 year storm, accounting for a 20% increase due to climate change. Thus, even under extreme storm conditions, the surface water can be attenuated without causing flooding downstream.

5.7 Residual Risk

As a result of the design measures detailed above in Section 5.6, there is a low residual risk of flooding from each of the surface water risks.

6. Groundwater

6.1 Source

Groundwater flooding occurs when the water table rises above the ground surface. This typically happens during periods with prolonged rainfall which exceeds the natural underground drainage system's capacity.

6.2 Pathway

The pathway for groundwater flooding is from the ground. Note that although groundwater flooding is typically considered to be when the water table rises above the ground surface, underground services and building foundations could also be affected by high water tables that do not reach the ground surface.

6.3 Receptor

The receptors for ground water flooding would be underground services, roads and the ground floor of buildings.

6.4 Likelihood

Geological Survey Ireland (GSI) produces a wide range of datasets, including groundwater vulnerability mapping. From the GSI groundwater vulnerability map, extracted below, the site lies within an area with typically moderate groundwater vulnerability.



Figure 8 | Extract of Groundwater Vulnerability Map

With the site falling within an area with moderate groundwater vulnerability, the likelihood of groundwater rising through the ground and causing potential flooding on site during prolonged wet periods is moderate.

6.5 Consequence

The consequence of ground water flooding would be some minor temporary seepage of ground water through the ground around the proposed buildings. Underground services could be inundated from high water tables. Therefore, the consequence of ground water flooding occurring at the proposed development is considered moderate.

6.6 Risk

With a moderate likelihood and moderate consequences of flooding due to groundwater, the risk is considered moderate.

6.7 Flood Risk Management

Finished floor levels have been set above the road levels, as described in Section 4.6, to ensure that any seepage of ground water onto the development does not flood into the buildings. In the event of ground water flooding on site, this water can escape from the site via the overland flood routing, also described in Section 4.6.

The buildings' design will incorporate suitable damp-proof membranes to protect against damp and water ingress from below ground level.

6.8 Residual Risk

There is a low residual risk of flooding from ground water.

7. Human/Mechanical Errors

7.1 Source

The subject site will be drained by an internal storm water drainage system, which discharges to the existing natural surface water network, the Mill Stream, which is a tributary of the Broadmeadow River, which in turn outfalls to the Malahide Estuary.

The internal surface water network is a source of possible flooding were it to become blocked.

7.2 Pathway

If the proposed private drainage system blocks this could lead to possible flooding within the private and public areas.

7.3 Receptor

The receptors for flooding due to human/mechanical error would be the ground floor levels of buildings, the roads and the open landscaped areas around the site.

7.4 Likelihood

There is a high likelihood of flooding on the subject site if the surface water network were to become blocked.

7.5 Consequence

The surface water network would surcharge and overflow through gullies and manhole lids. It is, therefore, considered that the consequences of such flooding are moderate.

7.6 Risk

With a high likelihood and moderate consequence, there is a high risk of surface water flooding should the surface water network block.

7.7 Flood Risk Management

As described in Section 4.6, finished floor levels have been designed to be above the adjacent road network, which will reduce the risk of flooding if the surface water network were to block. In the event of the surface water system surcharging, the surface water can still escape from the site by overland flood routing, as also described in Section 4.6, without causing damage to the proposed buildings.

The surface water network (drains, gullies, manholes, AJs, attenuation system) will need to be regularly maintained and where required cleaned out. A suitable maintenance regime of inspection and cleaning should be incorporated into the safety file/maintenance manual for the development.

7.8 Residual Risk

As a result of the flood risk management outlined above, there is a low residual risk of overland flooding from human / mechanical error.

8. Conclusions and Recommendations

The subject lands have been analysed for risks from tidal flooding from the Irish Sea and Broadmeadow River, fluvial flooding from Mill Stream & River Ward, pluvial flooding, ground water and failures of mechanical systems. *Table 4*, below, presents the various residual flood risks involved.

Source	Pathway	Receptor	Likelihood	Consequence	Risk	Mitigation Measure	Residual Risk
Tidal	Irish Sea (Malahide Estuary)	Proposed development	Extremely low	None	Extremely low	None	Extremely low
Fluvial	Broadmeadow River & Mill Stream	Proposed development	Low	Low	Extremely Low	Setting of floor levels & freeboard, overland flood routing, no localised low points	Extremely Low
Pluvial	Private & Public Drainage Network	Proposed development, downstream properties, and roads	Ranges from high to low	Moderate	Ranges from high to low	Appropriate drainage, SuDS, and attenuation design, setting of floor levels, overland flood routing	Low
Ground Water	Ground	Underground services, ground level of buildings, roads	Moderate	Moderate	Moderate	Appropriate setting of floor levels, flood routing, damp proof membranes	Low
Human/ Mechanical Error	Drainage network	Proposed development	High	Moderate	High	Setting of floor levels, overland flood routing, regular inspection of SW network	Low

Table 7 | Summary of the Flood Risks from the Various Components

As indicated in the above table, the various sources of flooding have been reviewed, and the risk of flooding from each source has been assessed. Where necessary, mitigation measures have been proposed. As a result of the proposed mitigation measures, the residual risk of flooding from any source is low.

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UK and Ireland Office Locations

